

1 **Fracture properties of GGBFS-blended fly ash**
2 **geopolymer concrete cured in ambient**
3 **temperature**

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13 **ABSTRACT**

14 Fracture characteristics are important part of concrete design against brittle failure. Recently, fly
15 ash geopolymer binder is gaining significant interest as a greener alternative to traditional ordinary
16 Portland cement (OPC). Hence it is important to understand the failure behaviour of fly ash based
17 geopolymers for safe design of structures built with such materials. This paper presents the
18 fracture properties of ambient-cured geopolymer concrete (GPC). Notched beam specimens of
19 GPC mixtures based mainly on fly ash and a small percentage of ground granulated blast furnace
20 slag (GGBFS) were subjected to three-point bending test to evaluate fracture behaviour. The effect
21 of mixture proportions on the fracture properties were compared with control as well as OPC
22 concrete. The results show that fracture properties are influenced by the mixture compositions.
23 Presence of additional water affected fracture properties adversely. Fracture energy is generally
24 governed by tensile strength which correlates with compressive strength. Critical stress intensity
25 factor varies with the variation of flexural strength. Geopolymer concrete specimens showed
26 similar load-deflection behaviour as OPC concrete specimens. The ambient cured GPC showed
27 relatively more ductility than the previously reported heat cured GPC, which is comparable to the
28 OPC specimens. Fly ash based GPC achieved relatively higher fracture energy and similar values
29 of K_{IC} as compared to those of OPC concrete of similar compressive strength. Thus, fly ash based
30 GPC designed for curing in ambient condition can achieve fracture properties comparable to
31 normal OPC concrete.

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1 **Keywords:** *Ambient curing, fly ash geopolymer, fracture energy, stress intensity*
2 *factor.*

3 **1 Introduction**

4 Fracture properties such as fracture toughness and fracture energy of a material
5 are used to describe the formation and progress of cracks in members made of that
6 material. Furthermore, continuous improvements of binders are improving
7 concrete behaviour approaching that of a homogeneous material (Juenger et al.
8 2011). Though concrete is generally referred as a brittle material, it actually
9 differs from an ideal brittle material in many aspects. In modern fracture
10 mechanics concrete is considered as a quasi-brittle material exhibiting a post-peak
11 softening behaviour which lies between a brittle and a ductile material behaviour.
12 Increased ductility can be provided by increasing the fracture energy by careful
13 choice of the constituent materials (Bharatkumar et al. 2005; Trussoni et al. 2013)
14 or by reinforcing the matrix with fibres (Deepa Raj et al. 2013).

15 Fracture characteristics of concrete are influenced by the material properties such
16 as strength, mixture constituents, and types of aggregate used, and the maturity of
17 concrete. The size of the specimens is also important factor affecting fracture
18 properties (Gettu et al.1990). Darwin et al. (2001) found a small variation of
19 fracture energy of OPC concrete when water-cement ratio varied between 0.25 to
20 0.45 at any age from 7 days to 180 days. For concretes at least five days old,
21 fracture energy is independent of compressive strength, w/cm, and age. Fracture
22 energy of concrete is governed principally by the properties of the coarse
23 aggregate, with higher strength aggregates (basalt) producing concretes with
24 higher fracture energies. In contrast, Gettu et al. (1990) found that, as the strength
25 increases, the fracture energy and the fracture toughness of concrete also increase.

1 In another study on high performance concrete (HPC), the results showed that
2 there is a reduction in the fracture energy due to addition of fly ash or slag
3 (Bharatkumar et al. 2005).

4 Geopolymer binder is a relatively new material that requires extensive research
5 into various properties to ensure its suitability for structural applications. The
6 binder is produced by chemical reaction of an aluminosilicate material such as fly
7 ash, blast furnace slag or metakaolin with an alkali (Juenger et al. 2011;
8 Davidovits 2008). Since geopolymers utilise by-products as the principle source
9 material, the binders are considered as a low CO₂-emitting alternative of Portland
10 cement (Yang et al. 2013). Low-calcium fly ash geopolymer concretes cured in
11 high temperature have been extensively researched for the last two decades and
12 are reported to have good mechanical properties (Wallah and Rangan 2003;
13 Fernandez-Jimenez et al. 2007). The structural properties of heat-cured fly ash
14 geopolymer concrete were reported to be similar or superior to that of OPC
15 concrete (Sumajouw et al. 2005; Sarker 2011; Sarker et al. 2013). However,
16 reports on the structural behavior of ambient-cured geopolymer concrete are
17 scarce in literature.

18 Sarker et al. (2013) reported fracture properties of heat cured fly ash based
19 geopolymer concrete and compared with OPC concrete. They found higher peak
20 load in the geopolymer than the OPC concrete of similar strength grade. The heat-
21 cured geopolymer concrete specimens were found to be more brittle than the OPC
22 concrete specimens. Fracture energy was found to be similar in both types of
23 concrete for similar compressive strengths. The results suggest that the different
24 fracture behaviour of geopolymer concrete is mainly because of its higher tensile
25 and bond strengths than OPC concrete of the same compressive strength.

1 Pan et al. (2011) reported the results of experimental research on fracture
2 properties of heat-cured fly ash based geopolymer concrete and paste with various
3 mix parameters. The results indicate that the fracture energy and elastic modulus
4 of geopolymer paste and concrete are lower than those of OPC paste and concrete.
5 The tensile strength of geopolymer paste and concrete is higher than that of OPC
6 paste and concrete. The characteristic length of the geopolymer concrete was
7 approximately three times less than that of ordinary Portland cement (OPC)
8 concrete. The geopolymer concrete exhibited higher brittleness than its OPC
9 counterpart.

10 Deepa Raj et al. (2013) worked on the fracture properties of fibre reinforced fly
11 ash based geopolymer concrete. The study concluded that the load carrying
12 capacity, deflections and crack mouth opening deflection of geopolymer are more
13 than those of OPC concrete at ultimate stage. Fracture energy and fracture
14 toughness are found to be greater than the OPC concrete.

15 These studies reported the properties of fly ash based geopolymers cured in
16 elevated temperature. High curing temperature and the activation method of the
17 aluminosilicate source material played the most important roles on the fracture
18 properties of geopolymers in these studies. However, the fracture behaviour of fly
19 ash based geopolymer concrete cured in ambient condition has not been studied.
20 Since the curing method has a significant influence on development of the
21 hardened properties, it is necessary to study the fracture behaviour of low-calcium
22 fly ash geopolymer concrete cured at ambient condition. Curing in normal
23 ambient condition will also help reduce the cost and energy associated with heat
24 curing. Therefore, this study aimed to investigate the fracture properties of low
25 calcium fly ash based geopolymer concrete mixtures suitable for curing in

1 ambient condition. Geopolymer concrete mixtures were produced using fly ash as
2 the principle binder and including a small percentage of GGBFS in order to
3 improve the setting and hardening properties of the mixtures at the early ages
4 (Nath and Sarker 2014). Fracture properties such as load-deflection behaviour,
5 fracture energy and critical stress intensity factor were determined from the test
6 results. Effects of the mixture variables were evaluated by comparing these
7 properties for different geopolymer concrete and those of the OPC concrete.

8 **2 Experimental program**

9 **2.1 Materials**

10 Geopolymer binder was prepared with a Class F fly ash (ASTM C 618), collected
11 from a West Australian power plant. A commercially available ground granulated
12 blast furnace slag (GGBFS) was added as a small part of the binder. The chemical
13 compositions and loss on ignition of fly ash and GGBFS are shown in Table 1.
14 The alkaline liquid used was a mixture of sodium hydroxide (NaOH) and sodium
15 silicate (Na_2SiO_3) solutions. Sodium hydroxide solution of 14 Molar
16 concentration was prepared by mixing 98-99% pure NaOH pellets with normal
17 tap water. Sodium silicate solution with SiO_2 to Na_2O mass ratio of 2.61 ($\text{SiO}_2 =$
18 30.0% , $\text{Na}_2\text{O} = 11.5\%$ and water = 58.5%) was used. Locally available natural
19 sand was used as fine aggregate, and coarse aggregates were a combination of
20 crushed granite with nominal maximum sizes of 7 and 10 mm. The maximum
21 coarse aggregate size of 10 mm was selected to meet the specification
22 recommended by RILEM for small size of specimens (RILEM TC 50-FMC 1985).
23 A superplasticiser (Rheobuild 1000) was used to improve workability. A General

1 purpose ordinary Portland cement (OPC) conforming to Australian standard (AS
2 3972) was used for the OPC concrete mixture.

3 **2.2 Mixture proportions**

4 Concrete mixtures were proportioned to investigate the fracture properties of
5 ambient cured geopolymer. Mix variables included the amount of GGBFS as a
6 replacement of fly ash and the amount of alkaline liquid. Five geopolymer
7 concrete (GPC) and one OPC concrete mixtures were used in the experimental
8 work. The mixture proportions of concretes are presented in Table 2. Two sets of
9 mixtures were designed for varying amount of alkaline solution as 40% and 35%
10 of the total binder. The first set of mixtures were designated as A40 S00, A40 S10
11 and A40 S15 which contained 40% alkaline liquid and 0%, 10% and 15% GGBFS
12 respectively. The other set of mixtures contained 35% alkaline liquid and either
13 0% or 10% GGBFS. The mixtures are designated as A35 S00 and A35 S10
14 respectively. The ratio of Na_2SiO_3 to NaOH solution in the alkaline liquid was
15 kept constant as 2.5 for all the geopolymer mixtures.

16 The slump test was conducted to measure workability of the mixtures. Mixtures
17 having 40% alkaline liquid achieved more than 200 mm slump after mixing which
18 is similar to that obtained in our previous study (Nath and Sarker 2014). Mixtures
19 having 35% alkaline liquid generally show lower workability. Hence, additional
20 water and superplasticiser were used in the mixtures having 35% alkaline liquid to
21 achieve similar slump values of mixtures having 40% alkaline liquid.

22 **2.3 Casting and curing of the test specimens**

23 The beam specimen for fracture test was 600 mm in length and 100×100 mm in
24 cross section. The geometric dimensions of all the test specimens were kept

1 constant. A 25 mm deep notch was cast at the mid-section of the beam. Different
2 notch depths were used for fracture tests by different researchers (Gettu et al.
3 1990; Sarker et al. 2013; Pan et al. 2011) in literature. However, previous studies
4 conducted with different notch-depth ratio revealed comparable results as long as
5 the specimen size remained constant. In this study a notch-depth ratio of 0.25 was
6 used after some trials with 0.25 and 0.50. The smaller ratio of 0.25 was selected to
7 reduce the chance of accidental cracking during de-moulding and handling of the
8 test specimens. It also ensured the ligament area sizable enough to observe the
9 crack propagation in the concrete. The mould was designed to facilitate carving
10 the notch while casting the specimen. Companion cylinder and prism specimens
11 were cast for compressive strength, modulus of elasticity and flexural strength
12 tests.

13 The geopolymer mixtures were mixed in a laboratory pan-mixer. The premixed
14 alkaline liquid was added gradually to the dry mixture of aggregates and binders
15 (fly ash and GGBFS). The mixing was then continued for 4 to 6 minutes until a
16 consistent mixture was obtained. The fresh concrete mixture was cast in the
17 moulds in two layers. Each layer was compacted using a vibrating table. The
18 moulds with finished concrete specimens were moved to the curing room (18-23
19 °C and $70 \pm 10\%$ RH) immediately after casting. After 24 hours, the specimens
20 were de-moulded and stored in the same condition (18-23°C and $70 \pm 10\%$ RH)
21 until tested. Note that the specimens were not subjected to heat curing. The GPC
22 specimens without GGBFS (A40 S00 and A35 S00) were de-moulded three days
23 after casting, because of the long setting time required for these mixtures.

24 To compare with the properties of GPC mixtures, an OPC concrete mixture was
25 designed in accordance with the ACI guideline (ACI 2011.1-91). The OPC

1 concrete specimens were immersed in water after de-moulding on the day after
2 casting. The specimens were removed from water after 28 days and stored in the
3 same condition as the geopolymer samples until tested.

4 **2.4 Test methods**

5 Three-point bending test was conducted to determine fracture properties of the
6 concrete specimens following the RILEM guidelines (RILEM TC 50 – FMC
7 1985). The beam was simply supported over a span of 500 mm with the notched
8 face down and a single point load was applied at the centre of the beam (Figure
9 1). To reduce the effect of friction between the loading platen and the specimen,
10 ball bearing support apparatus were used in accordance with the ASTM
11 (C78/C78M-10e) standard. The test was conducted using a closed-loop universal
12 testing machine (Instron Servo Control machine). The specimen was loaded to
13 induce a vertical mid-section deflection at a rate of 0.5 mm/min. This loading rate
14 was selected after several trial tests to ensure the maximum load is reached within
15 30–60 seconds as recommended in the RILEM guidelines. The load and vertical
16 deflection data were recorded by an automatic data acquisition system at rate of
17 100 readings per second. The accuracies of the load and deflection data were to
18 0.001 kN and 0.001 mm respectively. Load was applied until complete failure of
19 the specimen. Three identical specimens were tested for each of the mixtures at
20 both 28 and 90 days. Since the strength of ambient-cured geopolymers continue to
21 develop beyond 28 days, the test was extended to 90 days in order to understand
22 the influence of age on the fracture properties.

23 Compressive strength test was conducted at the age of 28 and 90 days. Cylinder
24 specimens (100 mm diameter and 200 mm height) were tested at a loading rate of
25 0.33 MPa/s. Testing for flexural strength was conducted following the Australian

1 Standard (AS 1012.11-2000). The test involves beam specimens of dimensions
2 100 mm × 100 mm × 400 mm loaded in pure bending to fail due to flexural stress.
3 Modulus of elasticity of concrete samples was determined according to the ASTM
4 standard (ASTM C 469/C 469M - 10).

5 **2.5 Evaluation of fracture parameters**

6 After the test, the load-deflection graph was plotted with the recorded data. The
7 fracture energy (G_F) was calculated by the work of fracture method using
8 Equation 1 given in the RILEM guidelines (RILEM TC 50-FMC 1985). It is the
9 summation of the work done by the external load calculated by the area under the
10 load-deflection curve, (W_o) and work done by self-weight of the beam.

$$11 \quad G_F = \frac{W_o + mg \delta_o}{A_{lig}} \quad (1)$$

12 where W_o = area under the load-deflection curve (N-m), m = mass of the beam
13 between the support (kg), g = acceleration due to gravity (9.81 m/s²), δ_o = the
14 deflection at the final failure of the beam (m) and A_{lig} = area of the ligament (m²).

15 The experimental values of fracture energy were compared with some established
16 model equations proposed in the literature for OPC concrete. The CEB-FIP
17 committee recommended a prediction formula as given by Eq. 2 (Bazant and Becq-
18 Giraudon 2002).

$$19 \quad G_F = (0.0469s^2 - 0.5s + 26) \left(\frac{f'_c}{10} \right)^{0.7} \quad (2)$$

20 where, s = maximum aggregate size (mm), f'_c = compressive strength of concrete
21 (MPa)

1 Another equation (Eq. 3) was proposed by Bazant and Becq-Giraduon (2002)
 2 following a statistical analysis of 238 test data on fracture energy of OPC concrete
 3 of strength and age varying in a wide range. The coefficient of variation of the test
 4 to prediction ratios of the results by this equation was found to be 29.9%. The
 5 equation takes into account the compressive strength, maximum aggregate size
 6 and the water to cement ratio of the concrete. Since geopolymer concrete is
 7 different from the usual mixture of OPC concrete, the term liquid to binder ratio
 8 of geopolymer mixture, as shown in Table 2, was used as equivalent to the water
 9 to cement ratio of OPC concrete. The liquid content was calculated by adding the
 10 amount of NaOH solution, Na₂SiO₃ solution and any extra water. Total binder
 11 content includes the binders such as fly ash and GGBFS.

$$12 \quad G_F = 2.5 \alpha_o \left(\frac{f'_c}{0.051} \right)^{0.46} \left(1 + \frac{s}{11.27} \right)^{0.22} \left(\frac{w}{c} \right)^{-0.30} \quad (3)$$

13 where, α_o is aggregates shape factor (1 for round aggregates, 1.44 for angular or
 14 crushed aggregates), and w/c is the water to cement ratio of the OPC concrete.

15 The critical stress intensity factor (K_{IC}) was calculated using Equation 4 (Peterson
 16 1980). It is also known as fracture toughness and relates to the peak load and the
 17 geometric dimensions of the beam.

$$18 \quad K_{IC} = \frac{3P_{max}l}{2bd^2} \sqrt{a_o} (1.93 - 3.07A + 14.53A^2 - 25.11A^3 + 25.8A^4) \quad (4)$$

19 where, P_{max} = the peak load, l = the span of beam, b = the width of beam, d = the
 20 depth of beam, a_o = the depth of the notch and $A = a_o/d$.

1 **3 Results and discussion**

2 **3.1 Mechanical properties**

3 **3.1.1 Compressive strength**

4 Mechanical properties of all the mixtures were determined at the same time when
5 the three-point bending tests were conducted. Results of compressive strength,
6 flexural strength and modulus of elasticity are given in Table 3. The compressive
7 strength and modulus of elasticity are also plotted in Figure 2. The 28-day
8 compressive strength of the geopolymer concretes of this study varied from 25
9 MPa to 46 MPa and increased up to 53 MPa at 90 days. It is clear from the results
10 that the compressive strength enhanced significantly with the increase of GGBFS
11 content in the mixtures designed with 40% alkaline liquid (Figure 2). The
12 improvement of strength due to inclusion of GGBFS followed the same trend as
13 reported in previous study (Nath and Sarker 2014). The increase of strength was
14 also observed when the alkaline liquid was reduced to 35% and no extra water
15 was added (mixture A35 S00). However, mixture A35 S10 showed lower strength
16 than that having 40% alkaline liquid and same additive content of 10% GGBFS
17 (A40 S10). This reduction in strength is caused by the addition of extra water
18 along with superplasticiser in the mixture A35 S10 (Table 2). The additional
19 water reduced the concentration of alkaline liquid which eventually decreased
20 strength. The geopolymer mixtures containing 35% alkaline liquid with or without
21 10% GGBFS resulted in similar compressive strengths. This result implies that
22 the inclusion of a small amount of GGBFS such as 10% can balance the negative
23 effect of additional water on the strength development of geopolymer concrete.

1 **3.1.2 Flexural strength**

2 It can be seen from Table 3 that flexural strength of geopolymer concrete cured in
3 ambient temperature increased with age for all the mixtures. This is similar to the
4 development of compressive strength with age, as observed in Figure 2. Also,
5 flexural strength increased with the inclusion of GGBFS content up to 10%. A
6 slight decline in the flexural strength was observed for increasing the GGBFS
7 content to 15%. From Table 3 it can be seen that, mixture A35 S10, having
8 additives and extra water in the mixtures, achieved about 30% less flexural
9 strength than that of control geopolymer A35 S00 which had no water added.
10 While mixtures having 35% alkaline liquid only with no extra water achieved
11 higher compressive and flexural strength, addition of water with 35% alkaline
12 activator in concretes having GGBFS have decreased flexural strength. This
13 indicates that the presence of extra water adversely affected flexural tensile
14 strength of ambient cured geopolymer concrete.

15 When compared with OPC concrete, geopolymer concrete of similar strength
16 (A40 S10) exhibited higher flexural tensile strength than the OPC concrete (Table
17 3). The result is consistent with that reported for both heat cured (Hardjito 2005;
18 Rangan 2007) and ambient cured geopolymer concretes (Deb et al. 2014).

19 **3.1.3 Modulus of elasticity**

20 Generally, the value of modulus of elasticity varied with the compressive strength
21 and no significant difference is observed due to variation of the mixture
22 proportions. Similar trends were observed at the ages of 28 days and 90 days, as
23 shown in Figure 2. Geopolymer concretes cured in elevated temperature generally
24 reported to have low modulus of elasticity as compared to that of OPC concrete of
25 the same compressive strength (Fernandez-Jimenez et al. 2006; Sofi et al. 2007).

1 The ambient cured geopolymer concretes of this study also showed similar trends.
2 It can be seen from Table 3 that the modulus of elasticity of geopolymer concretes
3 are relatively less than the OPC concrete of similar compressive strength. For a 40
4 MPa concrete, while OPC mixture had modulus of elasticity of 30.6 GPa, similar
5 grade geopolymer concrete (A40 S10) achieved a modulus of elasticity of 22.6
6 GPa at 28 days. This is about 24% less than OPC concrete at 28 days and the
7 difference increased to 29% after 90 days. In a previous study, heat cured
8 geopolymer concrete showed about 22% less modulus of elasticity than OPC
9 concrete (Pan et al. 2011).

10 **3.2 Fracture properties**

11 **3.2.1 Load-deflection behaviour**

12 Typical load deflection patterns of fly ash geopolymers containing GGBFS along
13 with 40% alkaline solution, and the OPC concrete are presented in Figure 3. The
14 initial non-linearity of the load–deflection curves were corrected to eliminate
15 distortions caused by the deformation of the specimen at the supports and
16 adjustments at contact planes (Sarker et al. 2013; RILEM TC 50-FMC 1985). Figure
17 3 shows the load deflection diagrams of different specimens at 28 days and 90
18 days. As usual, the curve showed a linear upward slope until the load reached
19 cracking limit of the material. It can be seen from the graphs that the slope of the
20 pre-peak curve generally increased for the specimens having higher strength than
21 control concrete (A40 S00). After reaching peak load, the crack initiates which
22 results in a downward post peak curve. The slope of post-peak part of the curve
23 reflects the property of the cracked specimen until breaking. The curvature of post
24 peak curve varied depending on the ductility of the material. The slope gradually

1 decreased with the increase of the compressive strength of the geopolymer
2 concretes. This indicates reduced ductility of the specimens with increasing
3 compressive strength. The control specimen of A40 S00, which had lowest
4 strength, showed greater stretch of the post peak curve before complete failure.
5 The load-deflection curves of geopolymer concretes are of similar shape as that of
6 OPC concrete. However, the pre-peak slopes of the curve appeared slightly
7 steeper in case of geopolymer specimens. This implies that OPC concrete of
8 similar grade tend to deflect more than geopolymer concretes before initiating the
9 crack. The post peak behaviour is almost similar for OPC and ambient cured
10 geopolymer concretes as indicated by the similar post-peak slopes. This is
11 different from that observed in the study of heat cured fly ash based geopolymer
12 concrete (Sarker et al. 2013). It is reported that, the post-peak load usually
13 dropped faster in the heat cured specimens than in the OPC concrete specimens
14 and showed a steeper slope of the post-peak curve. However, in this study of
15 ambient cured GPC, the post peak slope of load deflection diagram for both GPC
16 and OPC concrete gradually decreased rather than dropping sharply. Geopolymer
17 specimens showed relatively slightly greater stretch of deflection before ultimate
18 failure than OPC concrete at both 28 and 90 days. Hence it can be stated that
19 ambient cured GPC showed more ductile behaviour than heat cured GPC which is
20 comparable to that of OPC concrete.

21 **3.2.2 Peak load**

22 Peak load as a fracture property indicates the maximum load that is required to the
23 separate the surfaces involving in the crack which takes place across an extended
24 crack tip, or cohesive zone (Dugdale 1960). Peak loads of all the specimens are
25 given in Table 4. The average peak loads of ambient cured geopolymer concrete

1 varied in the range of 2.7 to 4.5 kN at 28 days and 3.2 to 5.2 kN at 90 days. Figure
2 4 shows the variations of average peak load and the flexural strength of the
3 concretes. At both the ages of 28 and 90 days, the peak load varied in the same
4 way as flexural strength of the concrete. Among the geopolymer concretes,
5 mixture A35 S00 showed the maximum peak load as well as flexural strength.
6 The mixture composition mainly influenced the flexural strength which led to the
7 variation of peak load value in three-point bending tests. The OPC concrete has
8 less flexural strength than the geopolymer concrete of equal grade (A40 S10);
9 however, OPC concrete showed similar peak load as compared to similar grade
10 geopolymer concrete. Heat cured fly ash based geopolymer generally showed
11 higher peak load as compared to OPC concrete (Sarker et al. 2013).

12 **3.2.3 Fracture energy**

13 Fracture energy (G_F) was calculated by the work of fracture method (Eq. 1).
14 Results of three-point bending tests are presented in Table 4. Fracture energy of
15 the geopolymer concretes varied in the range of 150 - 232.8 N/m at 28 days and
16 172.4 - 250.4 N/m at 90 days. The mean fracture energy values of the geopolymer
17 and OPC concrete mixtures are compared in Figure 5. It can be seen that the
18 mixture proportions of geopolymer concrete have influenced the fracture energy.
19 Generally, the fracture energy of geopolymer concretes tends to increase with the
20 increase of GGBFS up to 10% in the mixture. Regardless of alkaline solution
21 content, mixtures having GGBFS as partial replacement of fly ash showed higher
22 fracture energy as compared to the control geopolymers (A40 S00 and A35 S00).
23 Comparing between the controls, mixture A35 S00, which was mixed with 35%
24 alkaline liquid and superplasticiser, showed relatively less fracture energy albeit
25 achieving higher compressive strength than mixture A40 S00. When water and

1 superplasticiser were added in addition to 35% alkaline solution in the mixture,
2 these affected the fracture characteristics. For instance, mixture A40 S10 achieved
3 higher compressive strength and fracture energy (223.1 N/m at 28 day) than those
4 of mixture A35 S10 (197 N/m at 28 days).

5 Fly ash based geopolymer concrete of similar compressive strength achieved
6 relatively higher fracture energy than OPC concrete. For instance, mixture A40
7 S10 achieved 26% more fracture energy at 28 days and mixture A40 S15 achieved
8 37% more fracture energy at 90 days as compared to OPC concrete. Sarker et al
9 (2013) found that the fracture energy of heat cured GPC tends to increase with
10 compressive strength at a higher rate than OPC concrete. The mean 28-day and
11 90-day fracture energy values of this study and previous results on heat cured
12 GPC (Sarker et al. 2013) are plotted against compressive strength in Figure 6. It is
13 apparent that fracture energy of the ambient cured GPC increase with the increase
14 of compressive strength regardless of the age. The rate of increase is, however,
15 similar to the OPC concrete. It can be noted from Figure 6 that the fracture energy
16 values of ambient cured GPC were higher than those of the heat cured GPC. This
17 is probably due to increased brittleness of heat cured specimens that caused abrupt
18 failure of the concrete after reaching the peak load. As observed in load-deflection
19 curves, the ambient cured specimens of this study showed a gradual post peak
20 progression (Figure 3) rather than abrupt failure and resulted in larger work done
21 (fracture energy).

22 Figure 6 also compares the fracture energy values obtained from experiment with
23 those calculated using the prediction equations proposed by CEB-FIP (Eq. 2) and
24 Bazant & Becq-Giraduon (2002) (Eq. 3). The experimental values were found to
25 be significantly higher than those predicted using the CEB-FIP equation, whereas

1 the equation of Bazant & Becq-Giraduon predicted values relatively closer to the
2 experiment. However, this equation, which was originally developed from the
3 results of OPC concretes, calculated lower values of fracture energy for
4 geopolymer concrete. As observed in this study fly ash based geopolymer
5 concretes generally show higher fracture energy than that of OPC concrete. Hence
6 the equation of Bazant & Becq-Giraduon (2002) can be used conservatively for
7 preliminary estimate of fracture energy of fly ash based GPC cured in ambient
8 condition.

9 **3.2.4 Critical stress intensity factor**

10 Figure 7 shows the variation of the critical stress intensity factor (K_{IC}) with
11 respect to compressive strength of the studied mixtures of geopolymer and OPC
12 concrete at 28 and 90 days of age. The value of K_{IC} showed a gradual increasing
13 trend with the increase of strength. The geopolymer mixtures followed similar
14 values of K_{IC} as OPC concrete of similar compressive strength. This is different
15 from that reported for heat cured geopolymer which showed higher K_{IC} values as
16 compared to OPC concrete (Sarker et al. 2013). Compressive strength of the
17 mixtures played a significant role on the fracture parameters. However, some of
18 the mixtures, especially those having 35% alkaline liquid, showed different trend
19 which is more related to flexural tensile strength. Hence the values of K_{IC} of
20 different mixtures have been compared to flexural strength (modulus of rupture)
21 in Figure 8. It is evident that the values of K_{IC} followed the same trend of flexural
22 tensile strength development for all of the mixtures. The critical stress intensity at
23 the crack tip is governed by the peak load of concrete, which is also related to
24 tensile strength.

1 From Table 4 it can be seen that, most of the geopolymer mixtures followed the
2 trend of increasing fracture energy with the increasing value of K_{IC} . However,
3 mixtures A35 S00, which has only fly ash as the binder, 35% alkaline solution and
4 no extra water in the mix, showed comparatively high K_{IC} but less fracture energy
5 at both ages of 28 and 90 days as compared to other control concrete A40 S00 and
6 concretes having GGBFS with fly ash. This is due to its (A35 S00) high flexural
7 tensile strength and increased brittleness. This suggests that the geopolymer
8 concretes which were mixed with 35% alkaline liquid only tend to produce
9 geopolymer gel with relatively low fracture resisting capacity, i.e. low fracture
10 energy but high K_{IC} .

11 **4 Conclusions**

12 This study investigated fracture behaviour of fly ash based geopolymer concrete
13 cured in ambient condition. Geopolymer concretes were prepared with mainly fly
14 ash as the binder and GGBFS as an additive. Fracture properties were investigated
15 by three-point bending test of notched beam specimens. The following
16 conclusions are drawn from the results of the study:

- 17 • Inclusion of GGBFS in fly ash geopolymer enhanced compressive strength.
18 Flexural strength increased when GGBFS was added up to 10% of total binder.
19 The flexural strength of geopolymer concretes was higher than the OPC
20 concrete of similar compressive strength. When water was added with 35%
21 alkaline solution to facilitate workability, it caused an adverse effect on the
22 compressive and flexural strengths.
- 23 • Modulus of elasticity of geopolymer concrete varied likewise as compressive
24 strength. No adverse effect on the modulus of elasticity was seen for the

1 presence of GGBFS in addition to fly ash in the mixture. Similar to heat cured
2 geopolymer concrete, the modulus of elasticity of ambient cured geopolymer
3 concrete was less than that of OPC concrete of the same compressive strength.

- 4 • The ambient cured GPC specimens showed similar load-deflection behaviour
5 to that of OPC concrete. The ambient cured GPC showed relatively more
6 ductility than the reported heat cured specimens, which is comparable to the
7 OPC specimens. The peak load initiating crack varied with the flexural strength
8 of concrete.
- 9 • Generally, the fracture energy and critical stress intensity factor increased with
10 the increase of compressive strength regardless of age. The values of K_{IC}
11 showed the same trend of flexural tensile strength. The fracture energy of
12 concrete having GGBFS as an additive to fly ash was higher than that having
13 fly ash only. Fly ash based GPC achieved relatively higher fracture energy and
14 similar values of K_{IC} as compared to those of OPC concrete of similar
15 compressive strength. Ambient cured GPC resulted in higher fracture energy
16 values than that of the heat cured GPC.
- 17 • Geopolymer concretes which were mixed with 35% alkaline solution only
18 tend to produce geopolymer gel with relatively low fracture resisting capacity,
19 i.e. low fracture energy but high K_{IC} . When water was added in the GGBFS-
20 blended mixture with 35% alkaline solution, similar compressive strength,
21 higher fracture energy as compared to control mixture.

22 Finally, the mixture proportion of geopolymer and curing condition has
23 significant influence on the fracture properties. Fly ash based geopolymer
24 concrete designed for curing in ambient condition can achieve fracture properties
25 comparable to normal OPC concrete.

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5 **6 Compliance with Ethical Standards**

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7 **Conflict of Interest:** The authors declare that they have no conflict of interest.

8

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Table 1: Chemical composition of fly ash and GGBFS.

	SiO ₂	Al ₂ O ₃	Fe ₂ O ₃	CaO	MgO	Na ₂ O	K ₂ O	SO ₃	P ₂ O ₅	TiO ₂	LOI*
Fly ash (%)	53.71	27.2	11.7	1.9	-	0.36	0.54	0.3	0.71	1.62	0.68
GGBFS (%)	29.96	12.25	0.52	45.45	-	0.31	0.38	3.62	0.04	0.46	2.39

* Loss on ignition

Table 2: Mixture proportions of GPC and OPC concrete (kg/m³)

Mix ID	Coarse aggregate			Binders			Alkaline solutions			Super-plasticizer	Water/solid (w/s)
	10 mm	7 mm	Sand	Fly ash	GGBFS	OPC	Na ₂ SiO ₃	NaOH	Water		
A40 S00	651	558	651	400	0	-	114.3	45.7	0	0	0.202
A40 S10	651	558	651	360	40	-	114.3	45.7	0	0	0.202
A40 S15	651	558	651	340	60	-	114.3	45.7	0	0	0.202
A35 S00	655.9	562.2	655.9	400	0	-	100	40	0	6	0.180
A35 S10	655.9	562.2	655.9	360	40	-	100	40	6	6	0.193
OPC	430.4	368.8	921.4	-	-	387.9	-	-	213.4	0	0.55*

* water/cement (w/c) ratio for OPC concrete.

Table 3: Strength and modulus of elasticity results

Mixture	Compressive strength, f_{cm} (MPa)		Flexural Strength $f_{ct,f}$ (MPa)		Modulus of Elasticity, E (GPa)	
	28 day	90 day	28 day	90 day	28 day	90 day
A40 S00	25.6	33.4	4.89	5.91	17.4	20.0
A40 S10	38.3	45.5	5.79	6.47	22.6	23.8
A40 S15	46.6	53.3	5.26	6.12	24.6	25.2
A35 S00	32.5	41.1	6.13	7.68	19.8	22.8
A35 S10	33.3	43.0	4.27	5.52	19.2	22.2
OPC	41.6	50.6	3.68	4.97	30.6	33.4

Table 4: Fracture properties of ambient cured fly ash based geopolymers and OPC concretes at 28 and 90 days.

Mix ID	28 days						90 days					
	Mean f_c (MPa)	P_{max} (kN)	G_F (N/m)	Mean G_F (N/m)	K_{IC} (MPa-mm ^{1/2})	Mean K_{IC} (MPa-mm ^{1/2})	Mean f_c (MPa)	P_{max} (kN)	G_F (N/m)	Mean G_F (N/m)	K_{IC} (MPa-mm ^{1/2})	Mean K_{IC} (MPa-mm ^{1/2})
A40 S00	25.6	2.73	157.7	156.3	15.9	15.7	33.4	3.27	227.1	206.4	16.6	19.4
		2.74	154.9		15.5			3.12	184.7		19.9	
		-	-		-			3.42	207.3		21.6	
A40 S10	38.3	3.72	191.6	223.1	22.5	23.1	45.5	3.69	214.0	234.6	22.0	21.9
		3.61	216.9		22.0			3.60	216.5		20.9	
		4.02	261.0		24.7			3.76	273.2		22.7	
A40 S15	46.6	3.61	231.0	201.3	22.8	21.7	53.3	3.67	201.8	250.5	23.1	25.6
		3.60	200.6		22.4			4.24	305.8		27.3	
		3.55	172.2		19.8			4.37	244.0		26.4	
A35 S00	32.5	4.89	168.1	150.0	28.8	26.6	41.1	5.90	179.4	172.4	30.8	29.5
		4.53	145.9		26.7			5.05	185.0		25.6	
		4.09	136.0		24.3			4.88	153.0		29.0	
A35 S10	33.3	3.33	197.7	197.0	20.6	20.3	43.0	4.12	207.5	175.4	24.0	21.2
		-	-		-			3.07	143.3		18.3	
		3.29	196.3		20.1			-	-		-	
OPC	41.6	3.66	182.3	177.1	21.9	21.5	50.6	4.27	172.9	182.9	25.9	26.1
		3.24	173.2		20.5			4.33	175.1		26.8	
		3.55	175.7		22.0			4.29	200.8		25.7	

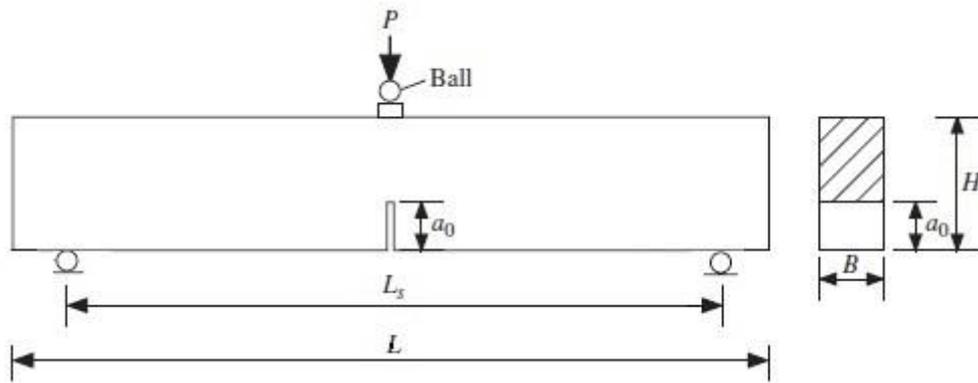


Fig. 1 Schematic diagram of three-point bending test

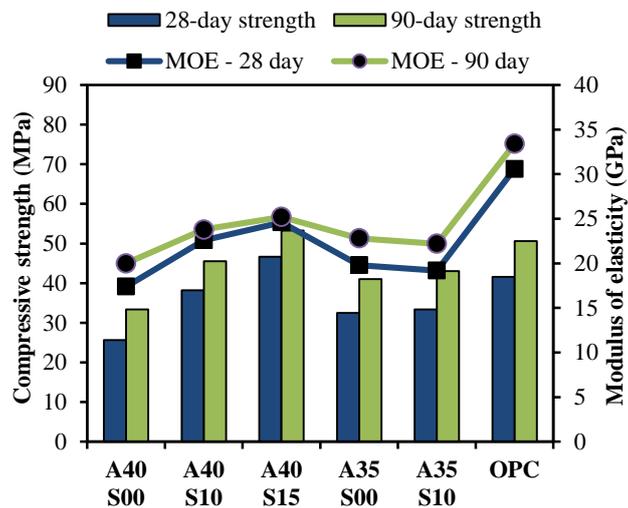
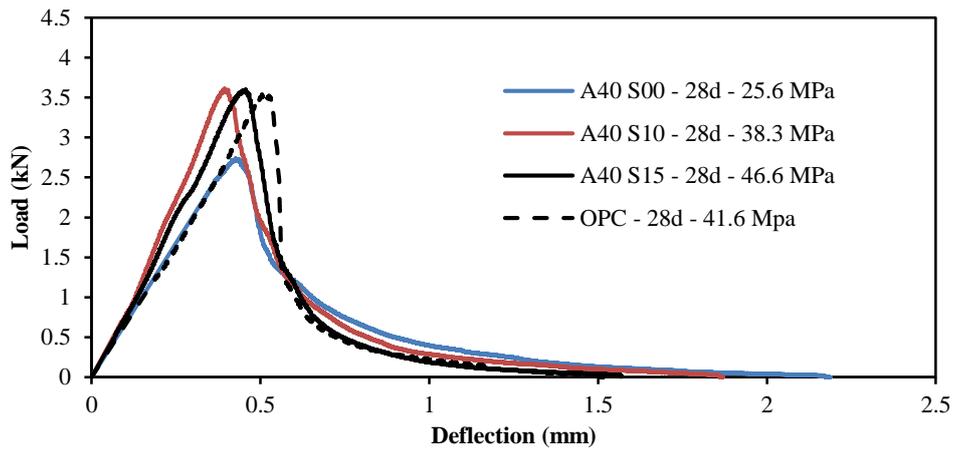
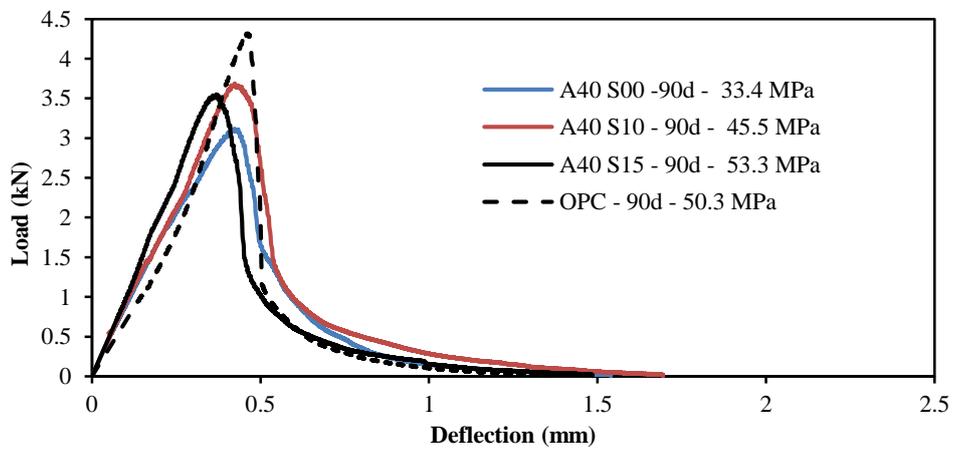


Fig. 2 Compressive strength and modulus of elasticity of geopolymer and OPC concretes



(a)



(b)

Fig. 3 Comparison of the load deflection diagrams of geopolymer concretes and OPC concrete at (a) 28 days and (b) 90 days

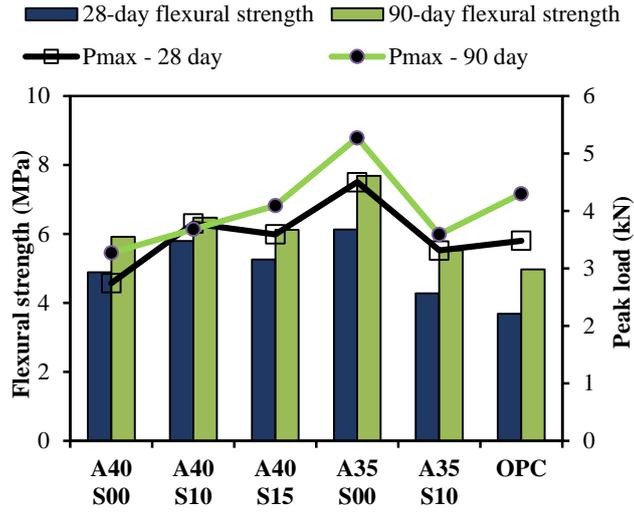


Fig. 4 Relationship of peak load to flexural strength at 28 day and 90 days

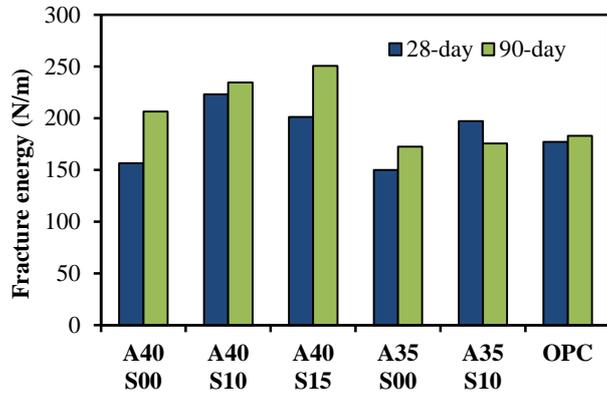


Fig. 5 Fracture energy of geopolymer and OPC concrete at 28 and 90 days

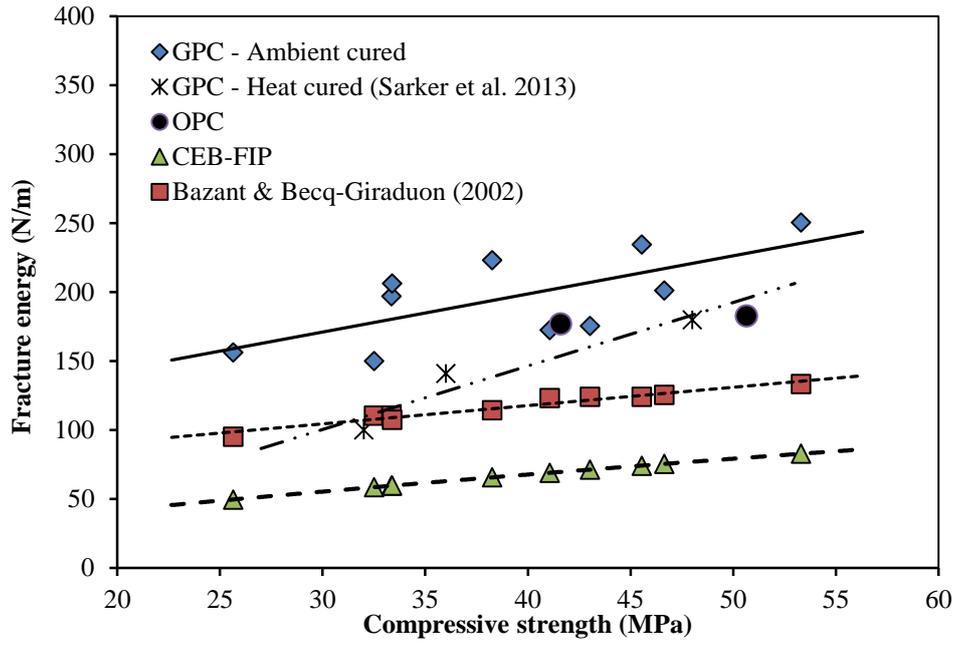


Fig. 6 Relationship of fracture energy and compressive strength

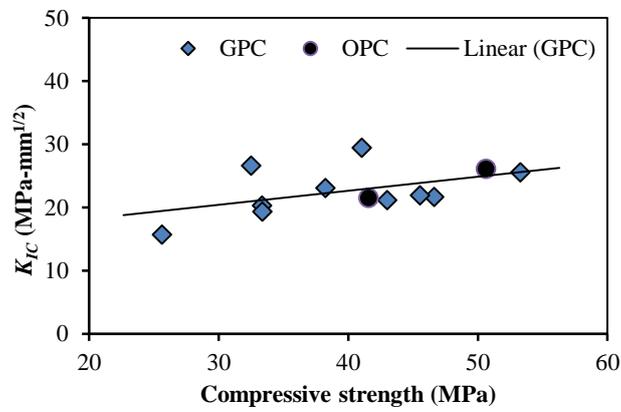


Fig. 7 Relationship of critical stress intensity factor with compressive strength

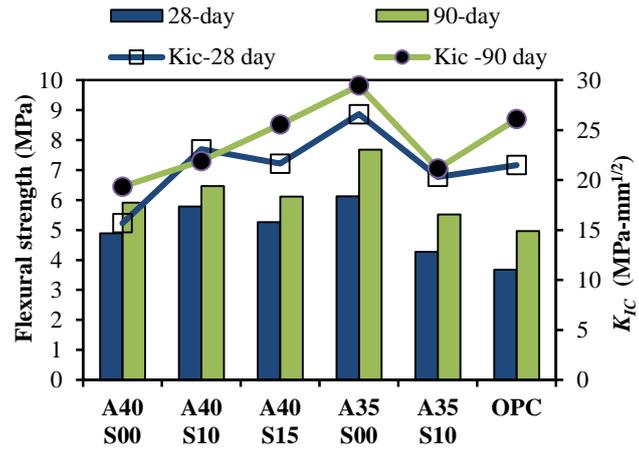


Fig. 8 Relationship of critical stress intensity factor with flexural strength.