

1           **Experimental and Numerical Study of Boundary and Anchorage**  
2           **Effect on Laminated Glass Windows under Blast Loading**

3                                   **Xihong Zhang<sup>1\*</sup>, Hong Hao<sup>2</sup>**

4           1.School of Civil, Environmental and Mining Engineering, the University of  
5           Western Australia, 35 Stirling Highway, Crawley WA 6009, Australia

6           2.Tianjin University and Curtin University Joint Research Center for Structural  
7           Monitoring and Protection, School of Civil and Mechanical Engineering, Curtin  
8           University, Kent St., Bentley WA 6102, Australia

9                                   \*email: xihong.zhang@uwa.edu.au

10       **Abstract**

11       Over the years extensive studies have been conducted to analyze the response of  
12       laminated glass panes under blast loading for personnel and property protection.  
13       The failure modes of glass windows in most of those studies are related to flexural  
14       bending of the glass panel. The problems of laminated glass failure at boundaries  
15       along window frames, as well as the influences of window frame constrain effect and  
16       the interlayer anchorage on the overall response of laminated glass panels are less  
17       examined. In this paper, experimental and numerical studies are carried out to  
18       examine the boundary conditions and interlayer anchorages of laminated glass  
19       windows on their responses under blast loadings. Blast tests were designed and  
20       conducted on window specimens with different frame bite depths, fixed or sliding  
21       boundaries and different interlayer anchorages. Numerical model of laminated glass  
22       windows is also developed. The accuracy of the numerical model in prediction of  
23       glass window responses is verified by field blast testing results. The validated  
24       numerical model is used to perform intensive simulations to study the window  
25       boundary conditions and interlayer anchorage measures on glass window responses  
26       to blast loadings. The results demonstrate that properly designed window frame and  
27       interlayer anchorage will increase the survivability of laminated glass windows under  
28       blast loadings.

29       **Keywords** laminated glass, field blast test, numerical analysis, anchorage

1 **1. Introduction**

2 Tragedies related to the hostile terrorist bombing attacks and accidental explosions  
3 are occasionally reported as news headlines throughout the world, e.g., the recent  
4 fuel tank explosion in Nanjing, China in June 2014, and the terrorist bombing attack  
5 in Oslo, Norway in 2011. Most post-event investigations of such incidents have cited  
6 the majority of human casualties and injuries were rather than by the air blast wave  
7 or the bomb container fragments themselves, but mainly by the shattered glass  
8 windows, fragments of walls and other objects which were not secured and were  
9 propelled towards the residents by the blast waves [1, 2]. Due to its relatively weak  
10 strength, glass windows in such incidents are especially fragile, and consequentially  
11 lead to enormous casualties. For better human protection against blasting loads, the  
12 development of blast-resistant windows has been being research topics of many  
13 researchers, manufacturers, security personnel and government officials all over the  
14 world.

15 Different techniques and materials are available to provide blast resistant glass  
16 windows, which include replacing low strength annealed glass by high strength  
17 thermally tempered glass or by laminated glass. Lin et al. conducted an intensive  
18 review on available window strengthening solutions [3]. Recent field blasting tests  
19 on monolithic glass windows found that by using thermally tempered glass, the blast  
20 resistant capacity of the glass windows can be effectively improved [4]. However,  
21 under large magnitude blast loads monolithic tempered glass windows rupture into  
22 numerous jagged shards which impose significant threats to the residents [5].  
23 Employing laminated glass panel for windows has proved itself through experiments  
24 and experiences of explosion incidents to effectively mitigate the risks of human  
25 injuries from ejecting glass fragments. Laminated glass consists of two or more glass  
26 plies bounded together by polymer interlayers such as Polyvinylbutyral (PVB) or  
27 SentryGlas<sup>®</sup> Plus (SGP, ionoplast produced by DuPont<sup>™</sup>) of different thicknesses.  
28 After glass crack under blast loading, the polymer interlayer will hold the glass  
29 splinters and continue to deform substantially as a membrane. In such a manner, the  
30 imposed blast energy will be dissipated by the laminated glass panel through large  
31 deformations.

1 The failure process of a laminated glass pane under blast pressure can be divided  
2 into the following five steps: (1) the entire laminated pane deforms elastically; (2)  
3 cracks are formed on the outer glass ply under tension; (3) cracks extend and occur  
4 on the inner glass ply; (4) the interlayer retains the cracked glass plies and continues  
5 to deform; (5) Rupture is formed on the interlayer. Zhang et al. studied the failure  
6 modes of laminated glass panes through numerical simulations [6]. It was found that  
7 if the laminated glass pane is clamped firmly, shear failure occurs on the interlayer  
8 along the boundary when it is subjected to impulsive load with significant reflected  
9 pressure in short duration; flexural bending failure is expected when it is under  
10 relatively long duration loading; and a combined shear and flexural failure will be  
11 formed on the PVB interlayer if it is under intermediate dynamic loading. Parametric  
12 studies have been carried out to study the influence of glass thickness, interlayer  
13 thickness and glass strength, etc. on the failure modes of glass panes [6, 7].

14 In analyzing the response of laminated glass windows to blast loads, the influence  
15 of boundary conditions is found to be significant. Larcher et al. [8] modelled a 1.0m  
16 × 0.8m laminated glass panel with different boundary conditions, i.e. fully fixed  
17 boundary, in-plane sliding boundary which restricted glass pane longitudinal  
18 movement in the direction of blast wave but allowed in-plane transitional sliding,  
19 and elastic boundary to model the supporting rubber strips between frame and glass.  
20 The numerical results showed the glass panes with different boundary conditions  
21 responded quite differently. A largest pane central deflection was found on the  
22 window with sliding boundary, while a smallest central deflection was resulted on  
23 the window with elastic boundary. A larger central deflection is more likely to cause  
24 interlayer rupture, which means the laminated pane with in-plane sliding boundary  
25 can be the most fragile. In Zhang et al.'s pressure-impulse analysis on 7.52mm thick  
26 laminated glass panels, the ultimate load bearing capacity of the laminated pane  
27 with pinned boundary was found to be about 15% more than that with fully fixed  
28 boundary condition [7]. By reducing the rotational restraints along the window  
29 boundary, a more flexible window system was achieved which exhibited better blast  
30 resistant performance. These analyses on window boundary conditions lead to the

1 possibility of adjusting the boundary conditions to further improve the blast resistant  
2 capacity of a laminated glass panel.

3 The ideal failure mode of laminated glass windows discussed above is not  
4 necessarily always achievable. In Hooper et al.'s full-scale field blasting tests on  
5 laminated glass windows [9], before tearing occurred on the PVB interlayer, the  
6 entire cracked laminated panes were pulled out of the window frame and pushed  
7 into the occupied area behind the windows. In other words, the failure of the  
8 window was mainly due to joint failure at the window boundary rather than the  
9 failure of the laminated glass pane itself. The bite depth, namely the embedment  
10 depth of the glass pane into the window frame, is believed to play an important role  
11 in the overall response of the laminated glass windows in face of blast loading.  
12 Morison mentioned that for laminated glass with 1.52mm thick or more interlayer a  
13 25-30mm deep bite is required to achieve the better blast loading resistance [10].  
14 Laboratory tests and field blasting tests on laminated glass panels reported recently  
15 provide more insights to the influence of window bite depth. For instance, Kranzer et  
16 al. [11] tested 7.52mm thick laminated glass panels fully clamped in 1100mm by  
17 900mm steel frames with 50mm bite depth. No boundary failure was observed on  
18 any of the four tested panes. In the airbag pendulum impact tests by Zhang and Hao  
19 [12] carried out on 600mm by 600mm laminated glass (various thicknesses) with  
20 30mm bite depth all around, pane slipping out of the frame was not observed either.  
21 These tests on laminated glass windows indicate that a properly designed bite depth  
22 is needed to prevent premature failure of pulling the laminated pane out from the  
23 window frame before the interlayer ruptures so as to achieve the full blast loading  
24 resistance capacities of the laminated glass panes.

25 To prevent the potential slippage failure along window boundary, interlayer  
26 anchorages have been introduced to stop the laminated panes from being easily  
27 pulled out of the frame. For example, in manufacturing laminated glass panes tails of  
28 PVB interlayer are left perimetally along the pane boundary, which are then  
29 clamped into the window frame to provide certain anchorage. Fixture bolts can also  
30 be applied along the frame at specific spacing, which further anchors the PVB tails to  
31 the window frame. Another measure introduced by US Air Force Research

1 Laboratory is called mechanical fixture bar method [13]. This method uses a doubly  
2 laminated glass pane which consists of three glass plies and two PVB interlayers. The  
3 ends of the PVB interlayers wrap around steel rods which are firmly mounted into  
4 the wall. When the laminated pane is under lateral loading, the steel rods will hold  
5 the PVB interlayers and stop the laminated pane from being pulled out of the  
6 window frame. The efficiencies of all these strengthening techniques have been  
7 proved individually by their respective developers, mainly by field blast tests.  
8 However, performance of the respective strengthening techniques applied to  
9 windows other than those tested are not clear. The advantages and disadvantages of  
10 each individual measure over the other are not known either. Therefore, study and  
11 analysis on these anchoring measures for general window systems are needed.

12 In this study, full-scale field blast tests were carried out on 7.52mm thick  
13 laminated glass panels fully clamped by two robust steel frames with 50mm bite  
14 depth all around. The blast pressures and the responses of the laminated panes were  
15 recorded by pressure sensor and mechanical Linear Voltage Differential Transducers  
16 (LVDT). High-speed cameras were used to assist monitoring the response of the  
17 panes with pre-plotted tracking dot matrix. A doubly laminated glass panel installed  
18 in an innovative sliding boundary frame system was also tested in comparison with  
19 the one installed in the fully fixed boundary frame to examine the performance of  
20 the proposed sliding boundary system in mitigating the blast loading effect.  
21 Numerical models of laminated glass were developed and calibrated with field blast  
22 testing results. Numerical simulations were then conducted to investigate the  
23 influences of boundary conditions, namely the fully fixed or sliding, bite depth, and  
24 the interlayer anchoring methods on responses of laminated glass windows to blast  
25 loads.

## 26 **2. Experimental Investigation**

### 27 2.1 Description of experiment setup

28 In the current work, laminated glass panes were tested with different weights of TNT  
29 at various stand-off distances in six shots. A reinforced concrete (RC) frame of  
30 approximately 3.4m by 3.2m by 2.0m (width by length by height) as illustrated in

1 Figure 1 was constructed with deep rooted independent footings to support the  
2 glass window specimens for the test. The testing block consisted of two individual  
3 cells. The back wall of the block was left open for high-speed cameras to monitor the  
4 deformation of the glass panes. In each shot, two glass panes were tested with  
5 designed charge detonated in front of the RC block. The glass window specimens  
6 were installed on the openings of the front wall using steel frames. The laminated  
7 glass panes were 1.5m x 1.2m in dimension. For the first five tests, the laminated  
8 panes constructed with two plies of 3mm thick annealed glass sandwiching a  
9 1.52mm thick PVB interlayer (Figure 2a). These five 7.52mm laminated glass panes  
10 were tested in pair with another five glass panes of the same sizes but different glass  
11 and interlayer thicknesses. The responses of the other five glass panes were used to  
12 evaluate other issues therefore not included in this article. The tested laminated  
13 glass panes were firmly clamped with steel frames as illustrated in Figure 3a. The  
14 window frame, as shown, consisted of a 20mm thick inner frame, which was fixed  
15 onto the front wall of the RC block using M24 bolts. The testing panes were placed  
16 on the inner frame, and then covered with a 10mm thick outer steel frame. The  
17 outer frame was fastened with care onto the inner frame using M12 bolts. Torque  
18 wrench was used to ensure an equal compression was applied to glass pane through  
19 these M12 bolts. During installation, plastic strips were inserted in the gaps between  
20 the inner and outer frames to avoid damaging glass pane when fastening the bolts.  
21 There was no clearance gap left between glass and the window frame. In this  
22 manner, a fully fixed boundary condition was created for the laminated glass  
23 windows to be tested. The bite depth of the frame is 50mm. No silicone or epoxy  
24 was squeezed between glass and the frame. Therefore there was no epoxy bond at  
25 the interface.

26 Besides the 7.52mm laminated glass panes described above, two doubly  
27 laminated glass panes which comprise of three layers of 6mm annealed glass  
28 sandwiching two 1.52mm PVB interlayers (Figure 2b) were also tested in pair to  
29 examine the effectiveness of a sliding boundary over the traditional fully fixed  
30 boundary for mitigating glass window damage to blast loads. In the test, one glass  
31 pane was supported with the fully fixed boundary as described above and another

1 one with the sliding boundary. As shown in Figure 3b, the sliding boundary frame  
2 consisted of the same inner and outer frames as in the fixed condition. An extra thick  
3 layer of plastic pad was placed in between the two frames. The testing glass panes  
4 were inserted into the gap and rested against the outer frame. After fastening the  
5 M12 bolts, a 50mm sliding distance was created for the glass panes to move freely in  
6 the direction of blast wave. When the blast wave acts on the windows, the  
7 laminated panes is able to slide in the direction of loading to mitigate part of the  
8 shock wave energy, which will reduce pane deflection, as well as the pulling out  
9 potential of the laminated pane from its frame. Using a doubly laminated pane  
10 instead of the single laminated one in this test is to increase the stiffness and  
11 strength of the glass pane, so as to avoid immediate pane failure before it slides.  
12 Therefore the effectiveness of using sliding boundary can be examined in the tests. It  
13 should be noted that in the current test, the glass pane was placed in the sliding  
14 boundary without any support. In practice, however, some elastic material with  
15 small stiffness might be used to support the glass pane, which will make the glass  
16 pane not exactly free sliding. Therefore the effectiveness of allowing glass pane to  
17 slide freely for blast energy absorption might not be fully achievable in practice.

18 The targets of the experimental tests are to measure the laminated glass pane  
19 deflections under different blast loadings, to monitor the failure process and to  
20 study the failure modes of the laminated panes at joints with window frames. A  
21 pressure transducer was installed on the front wall of RC block between the two  
22 glass windows to measure the blast pressure. LVDTs were fixed onto two steel  
23 frames behind the windows inside the RC block to record the central displacements  
24 of the glass panes. The transducers were wired through an amplifier to a portable  
25 data acquisition system, which was setup dozens of meters away and hidden behind  
26 a concrete bunker. The sampling frequency for data collection was set to be 0.5MHz.  
27 Two high-speed cameras (Fastcam SA3 Photron<sup>®</sup>) were placed at an angle behind  
28 each window outside the RC block, and were protected by two heavy steel bunkers.  
29 An 11-row by 9-column black dot matrix (100mm spacing) was plotted on each  
30 laminated glass pane before the test. With the tracking dot matrix, the two high-  
31 speed camera images could also be used to monitor the deformation and response

1 of each glass pane. The filming frequency of the high-speed cameras was setup to  
2 2kHz. The aperture of the lens and the exposure time were adjusted accordingly. In  
3 each test, the high-speed imaging process and the data acquisition for pressure and  
4 displacement were triggered by signals from external wires glued directly onto the  
5 charge.

6 Table 1 lists the information of the laminated glass panes presented in the  
7 current study. It should be noted that a total of 13 blast trials were carried out. This  
8 paper devotes to examining the influences of boundary conditions on laminated  
9 glass windows. Therefore only test 1, 2, 3, 4, 6 and 7 were presented here. In the  
10 first three tests (test 1-3), the 7.52mm laminated panes were fully instrumented with  
11 measured reflected pressure histories, central displacement histories, and recorded  
12 high-speed images. The recorded pressure and pane central displacement histories  
13 will be later used to validate the numerical model described in this paper. For test 4  
14 and 6, reflected pressure histories and the failure processes of the laminated panes  
15 were recorded. The failure modes of the laminated panes were used to analyze the  
16 influences of bite depth on window responses under different blast loads. The  
17 doubly laminated pane with sliding boundary was tested together with another  
18 identical pane with fixed boundary condition in test 7. LVDT was not installed in this  
19 test to avoid damaging the sensors owing to pane sliding. However, the responses  
20 were captured by the high-speed camera images.

## 21 2.2 Testing results

22 The experimental results from the full-scale blast tests are presented in this section.  
23 The recorded blast loads, glass pane failure processes, pane failure modes, and the  
24 central displacement histories are provided and analyzed.

### 25 2.2.1 Blast loads

26 The primary charge for the current tests was Trinitrotoluene (TNT). The TNT  
27 explosives were casted into cylinders with desired weights. A 5cm diameter hole was  
28 left in the centre for the RXD booster charge. Electric detonators were inserted into  
29 the axis of the booster charge. Figure 4 shows the reflected pressure recorded by the



1 pressure transducer for the first three tests (1-3). The time axis is aligned to the  
2 instance when shock front arrived at the glass windows. As shown in Figure 4a, in  
3 test 1 the detonation of 10kg TNT at 10m away resulted in substantial reflected  
4 pressures (about 121kPa) which dwindled to ambient quickly. Long duration  
5 negative pressures followed, which attenuated gradually. Table 2 summaries the  
6 reflected pressures recorded for both the positive phase and the negative phase. The  
7 recorded reflected pressures are integrated along the time axis to derive the  
8 reflected impulses. Estimations using Kingery-Bulmash equations are also provided  
9 to demonstrate testing consistency.

#### 10 2.2.2 Displacement histories and failure processes

11 Figure 4a-c show the glass pane central displacement time histories recorded by  
12 the LVDTs on the three 7.52mm laminated panes in test 1-3 together with their  
13 applied reflected pressure histories. For instance, as shown in Figure 4b the glass  
14 pane in test 4 responded to the air blast wave with a relatively gradual increase in its  
15 central displacement initially. As glass plies cracked, the central displacement began  
16 to increase quickly with a steeper slope over time. The interlayer membrane still  
17 held the cracked laminated glass pane together. A maximum deflection of about  
18 320mm was reached, after which the pane began to rebound. The measured  
19 displacement history ceased soon after it rebound because the probe of the LVDT  
20 debonded from the cracked glass ply.

21 Figure 5a shows the snapshots of high-speed camera images from the 7.52mm  
22 laminated glass pane in test 4. As shown, the laminated glass pane deformed under  
23 the air blast pressure and the back glass ply cracked at 2 ms (at  $t=17\text{ms}$ ) after the  
24 shock wave was applied onto the window. The pane reached its maximum deflection  
25 at 25ms, after which it began to rebound. The cracked laminated pane was pulled  
26 out along its boundaries during rebound at  $t=35\text{ms}$ . At  $t=55\text{ms}$  the laminated pane  
27 was totally pulled out of the frame. Figure 5b shows the high-speed camera images  
28 of the laminated pane in test 1. As shown, the laminated pane reached a maximum  
29 deflection at about 30 ms or about 15 ms after the blast wave arrived at the window  
30 which is consistent with the LVDT recording as shown in Figure 4a. The cracked

1 laminated pane rebounded, but joint failure did not occur. At 134ms the pane was  
2 still firmly clamped in the window frame without any sign of joint failure. It is to be  
3 noted that in test 1 the aperture of the high-speed camera mismatched with the  
4 light. As a result, over exposure occurred when the overwhelming light from  
5 detonation made glass crack not visible initially. Nevertheless, the high-speed  
6 camera images still provided information on how the laminated pane responded  
7 during the blast. The high-speed camera images show that both panes in test 1 and 4  
8 survived the positive phases of the blast load, the maximum deflections were  
9 reached without boundary failure, but the laminated pane in test 4 was pulled out of  
10 the window frame during rebound possibly due to the sustained negative pressure.

### 11 2.2.3 Failure modes

12 Figure 6 shows the failure modes of the tested 7.52mm laminated glass panes  
13 after the blast tests. It can be observed that glass plies of all the tested windows  
14 were badly shattered, and larger blast load leads to more severe damage of the  
15 same glass window as clearly observed in the damaged pane 3-1-1 with  $Pr^+=82\text{kPa}$ ,  
16 and pane 1-1-1 with  $Pr^+=121\text{kPa}$ . Moreover, PVB tearing was found on the laminated  
17 pane 1-1-1, but not in pane 3-1-1. Both panes remained in the window frame  
18 without boundary failure. Partial pulling-out failure was observed on pane 6-1-1  
19 under increased blast loading. As shown, this laminated pane was partly pulled out  
20 of the window frame along its two vertical and bottom boundaries. The pulled-out  
21 part of the laminated pane was outside the window frame facing the explosion  
22 centre, indicating the pane was pulled out during rebound by the negative phase  
23 blast pressure. Total pulling-out failure was found on the other two laminated glass  
24 panes, namely pane 2-1-1 and 4-1-1 owing to larger blast loadings in these two shots  
25 as given in Table 2. As shown in Figure 6d and e, the laminated pane was totally  
26 pulled out of the frame, and left on the ground in front of the window, indicating  
27 again the action of the negative phase blast pressure. The high-speed camera images  
28 shown in Figure 5a illustrate the pulling out process during the laminated pane  
29 rebound.

1 None of the 7.52mm laminated glass windows tested in the current blast trials  
2 experienced large interlayer tearing. In fact the interlayer still held most of the  
3 cracked glass fragments, indicating great performance of the PVB interlayer in  
4 mitigating the blast loading hazards from glass fragments. However, as shown in  
5 Figure 6, the cracked laminated glass panes could be partially or totally pulled out of  
6 the frame, which also imposes significant threats to people in the vicinity. The  
7 observed pulling-out failure was possibly because the glass in contact with the steel  
8 window frame was damaged during the positive blast loading phase owing to large  
9 blast pressure and window deformation. The crushed glass layer inside the frame  
10 resulted in a loss of contact of glass pane with the window frame. Therefore the  
11 glass panes were pulled out during the negative blast loading phase. Since falling  
12 glass pane is also hazardous and should be avoided, it is therefore important to  
13 understand such damage modes at the glass pane boundary and properly design the  
14 anchorage and window frame to prevent the pull-out damage of laminated glass  
15 windows under blast loading.

### 16 2.3 Comparison with previous testing data

17 The blast testing results presented above show that laminated glass pane could  
18 be sufficiently strong to resist blast loadings. In such cases, the damages related to  
19 glass fragmentation and PVB interlayer rupture do not occur, but damage at the  
20 glass pane boundary might happen that results in the pulling-out of glass pane from  
21 the window frame. To further examine this possible damage mode, previous field  
22 blast testing results on 7.52mm laminated glass windows conducted by other  
23 researchers are collected and analyzed in this section.

24 As mentioned above, Hooper and his colleagues tested 1.5m x 1.2m laminated  
25 glass with 25mm embedment [9]. Four blast trials with blast loads from various  
26 combinations of C4 charge weights and stand-off distances were conducted. Among  
27 Hooper et al.'s four tests, one laminated pane at the 152kPa peak reflected pressure  
28 and 461kPa-ms reflected impulse was considered severely damaged because the  
29 cracked laminated pane was totally pulled out from the clamping frame along all  
30 four sides and pushed into the testing room (Figure 7b). In comparison, pane 2-1-1 in

1 the current blasting test was subjected to blast loading of similar magnitude  
2 ( $P_r=169\text{kPa}$ ,  $I_r=476\text{kPa}\cdot\text{ms}$ ), and pulling-out failure also occurred along the window  
3 boundaries. However, as described above and shown in Figure 7a, instead of being  
4 pushed into the testing cell, the laminated pane was pulled out of the frame and  
5 sucked out of the testing cell. Comparing the recorded reflected pressure with that  
6 in Hooper's test, the current test has a slightly higher blast pressure and impulse.  
7 The high-speed camera images show that the glass pane survived the positive  
8 pressure phase, but was pulled out from its frame during the negative pressure  
9 phase. The reason for these different failure modes is probably due to the larger bite  
10 depth of pane 2-1-1 in the current study. Compared to the 25mm bite depth in  
11 Hooper's test, the 50mm bite in the current specimen provided greater resistance to  
12 hold the cracked laminated pane sliding into the room during the positive blast  
13 pressure phase, although the gripping effect of the frame bite was weakened as  
14 friction between glass and steel strips degraded when cracks extended through the  
15 glass in contact with the frame. On the other hand, as demonstrated by some  
16 researchers that larger pane deflection could be expected when the effect of  
17 negative pressure is superposed with the rebound of the laminated pane [14]. The  
18 amplified deflection during rebound led to the laminated pane being pulled-out of its  
19 frame in the current tests.

20 Figure 8 summarizes the maximum pane central deflections of laminated glass  
21 windows with different bite depths under various blast loadings obtained in the  
22 current study and reported by other researchers in literature. The reflected impulse  
23 is used as x-axis to show the magnitude of blast loads. Considering window size  
24 differences, the reflected impulses are normalized against window size (the square  
25 root of window area). In the x-axis, a and b stand for window length and width  
26 respectively. In Figure 8, the solid symbols indicate the tested panes failed with joint  
27 failure, while the open symbols represent those without joint failure. With 25mm  
28 bite depth, the four 1.5m by 1.2m laminated panes tested by Hooper et al. had a  
29 small bite over pane width ratio of  $25\text{mm}/1200\text{mm}=0.021$ . Under  $461\text{kPa}\cdot\text{ms}$   
30 reflected impulse, the laminated pane was pushed into the testing cell with joint  
31 failure because of the insufficient anchorage of the pane in the frame. Another pane

1 tested in [9] also experienced severe damage along its boundary when it was  
2 subjected to 391kPa-ms reflected impulse, but was not completely pushed out of its  
3 frame owing to the restraints at the four frame corners. In the current field blast test,  
4 with 50mm glass embedment into the frame, it had a higher bite depth over window  
5 width ratio of  $50\text{mm}/1200\text{mm}=0.042$ . The deeper bite provided higher anchorage  
6 against pulling-out failure. As shown above, under 395kPa-ms and 413kPa-ms  
7 reflected impulses, the laminated panes in the current test were firmly restrained in  
8 the frame despite large pane deformations. When subjected to higher blast loadings,  
9 i.e., 476kPa-ms reflected impulses, the 7.52mm laminated pane failed along its  
10 boundaries and was forced out of the window frame. However, due to the restraint  
11 effect of deep bite, this pane survived the positive phase blast loading, but was  
12 pulled out of the window frame during rebound. Kranzer et al. [11] also provided  
13 50mm bite to the laminated glass panes in their experimental tests of smaller  
14 window specimens of dimension 1100mm × 900mm. Because the ratio of bite depth  
15 over window width was higher ( $50\text{mm}/900\text{mm}=0.056$ ), in their blast tests, all four  
16 laminated panes were firmly held by the rigid window frame. No joint failure was  
17 found among the tested panes. Through the above comparison it can be concluded  
18 that bite depth to window dimension ratio plays an important role in preventing  
19 joint failure. Depending on the bite over pane width ratio, as well as the blast loading  
20 amplitude, the laminated glass window joint failure might happen although the PVB  
21 interlayer could survive the blast loads and keep the shattered glass fragments  
22 together. The failed window joints may result in the window pane being pushed into  
23 the room or sucked outside by the negative blast pressure. It is therefore important  
24 to properly design the anchorage to prevent the joint failure of laminated glass  
25 windows.

### 26 **3. Numerical Simulation**

27 To further investigate the effectiveness of glass pane anchorage on preventing joint  
28 failure of laminated glass windows, a three dimensional finite element model of  
29 laminated glass window is generated using the commercial software LS-DYNA.  
30 Detailed laminated glass windows including the steel window frames as described

1 above in the field tests are modeled numerically. The model is calibrated with field  
2 blast testing results. Extensive numerical simulations are then carried out with the  
3 verified model to study the influence of bite depth, and different interlayer  
4 anchoring retrofit measures on preventing joint failure.

### 5 3.1 Model description

#### 6 3.1.1 Model configuration

7 Figure 9a depicts the typical finite element model of the laminated glass panel  
8 with fully fixed steel frame. 8-node solid elements are adopted to model the  
9 windows. Each node has six degrees of freedom. Full integration is utilized. The  
10 laminated pane is 1.5m high by 1.2m wide with element size 5mm × 5mm in within  
11 the window plane. For the 7.52mm laminated pane, it consists of 2 layers of 3mm  
12 thick annealed glass and one layer of 1.52mm PVB interlayer. Each layer has two  
13 elements in the thickness direction (Figure 9b). For the doubly laminated pane  
14 described above in test 7, each of the three 6mm thick glass plies and the two  
15 1.52mm PVB interlayers are also meshed with two elements along the thickness  
16 direction. The window frame comprises a 20mm thick inner and a 10mm thick outer  
17 steel strip. The frame is also meshed with 5mm × 5mm × 5mm solid elements. Full  
18 integration is adopted in the numerical simulation. Blast load is applied on the  
19 surface of the outer glass layer (as demonstrated in Figure 9b). Considering  
20 symmetry, only one quarter of the window specimen is included in the model.  
21 Erosion is introduced to model glass crack and interlayer rupture. The mass of the  
22 deleted elements is retained so as to maintain mass conservation. Figure 9a and b  
23 illustrate the numerical model generated for the laminated glass window in the  
24 current field blast tests. Figure 9c-f show the numerical models built for laminated  
25 glass with bolt anchor and bar anchor which will be described in detail in the  
26 following sections.

#### 27 3.1.2 Convergence study

28 A mesh size sensitivity test is performed to determine the optimized element size.  
29 The number of element in the window thickness direction is kept the same to ensure  
30 the stress variation across the pane depth is captured. Five different planer mesh

1 sizes, namely 50mm, 20mm, 10mm, 5mm, and 2mm are used to model the  
2 laminated glass panel in convergence test. The maximum pane central deflection is  
3 chosen to check the simulation convergence. As shown in Figure 10, when simulating  
4 the 7.52mm laminated pane in test 3, the resulted maximum deflection converges  
5 with 5mm mesh. Further reducing the mesh size to 2mm does not lead to any  
6 significant variation on the numerical results, but it leads to substantial increase in  
7 the computational time. Therefore, the mesh size is chosen to be 5mm.

### 8 3.1.3 Material model

#### 9 *Glass*

10 Glass is a complex material. The failure of glass is brittle. Compressive test on  
11 annealed glass material found its ultimate compressive strength could be over 1GPa  
12 [15]. The theoretical tensile strength of glass crystal can even reach 21GPa [16].  
13 However, because of existing flaws on its surface during manufacture and service,  
14 annealed glass used for architectural windows normally fails between 8MPa to  
15 45MPa [8]. A Weibull distribution is often introduced by some researchers to  
16 describe the tensile strength of glass for design purposes. When dealing with glass  
17 under ballistic impact or under blast loading, it is normally treated as a brittle  
18 material with a damage model to describe the strength deduction due to damage  
19 [15, 17, 18].

20 Recent studies on annealed glass material properties have found glass to be a  
21 strain-rate sensitive material [19, 20]. The dynamic increment factors (DIF) have  
22 been concluded. As shown in Figure 11, both the compressive and tensile strengths  
23 of architectural annealed glass will be amplified when it deforms at high strain rates.  
24 The strength increment at high strain rate could be caused by either the true  
25 material strength increment or the structural confinement effect in high-speed  
26 impact tests. Brown attributed the increase in glass strength to the assumption that  
27 flaws and cracks take time to extend to form rupture [21]. More thorough study is  
28 needed to better understand dynamic glass material properties.

29 Based on previous studies on dynamic material properties of annealed glass  
30 material, the material constants of the popularly used Johnson Holmquist Ceramic

1 (JH2) material model are recently derived for architectural annealed glass. JH2 model  
 2 is a well-defined material model for ceramic and glass materials. It includes a  
 3 strength model, a damage model, strain-rate effect, and equation of state (EOS). The  
 4 strength of material is depicted by the following equation

$$\sigma^* = \sigma_i^* - D(\sigma_i^* - \sigma_f^*) \quad (1)$$

5 where  $\sigma_i^*$  is the normalized intact strength,  $\sigma_f^*$  is the normalized material strength  
 6 at fracture, and D is the damage scalar ( $0 \leq D \leq 1$ ).

7 The normalized intact strength and material strength at fracture with strain-rate  
 8 effect are given by

$$\sigma_i^* = A(P^* + T^*)^N(1 + C \ln \dot{\epsilon}^*) \quad (3)$$

9 and

$$\sigma_f^* = B(P^*)^M(1 + C \ln \dot{\epsilon}^*) \quad (4)$$

10 where A, B, C, M, N and T are material constants;  $P^*$  stands for the normalized  
 11 pressure ( $P^*=P/P_{HEL}$ ), where P is the actual pressure and  $P_{HEL}$  is the pressure at HEL.  
 12 Similarly,  $T^*$  is the normalized maximum tensile hydrostatic pressure ( $T^*=T/P_{HEL}$ ).  $\dot{\epsilon}^*$   
 13 is the actual strain rate over the reference strain rate ( $\dot{\epsilon}^* = \dot{\epsilon}/\dot{\epsilon}_0$ , where  $\dot{\epsilon}_0 = 1.0 \text{ s}^{-1}$ ).

14 The damage owing to glass fracture in the JH2 model is defined by

$$D = \sum \Delta \epsilon_p / \epsilon_p^f \quad (6)$$

15 where  $\Delta \epsilon_p$  is the plastic strain during a cycle of integration, and  $\epsilon_p^f$  is the plastic  
 16 strain to fracture under constant pressure P,

$$\epsilon_p^f = D_1(P^* + T^*)^{D_2} \quad (7)$$

17 where  $D_1$  and  $D_2$  are material constants.

18 The equation of state for glass under compression is expressed as

$$P = K_1 \mu + K_2 \mu^2 + K_3 \mu^3 + \Delta P \quad (8)$$



1 where  $K_1$ ,  $K_2$ ,  $K_3$  are constants, and  $K_1$  is the material bulk modulus.  $\mu = \rho / \rho_0 - 1$ , in  
2 which  $\rho$  is the current density and  $\rho_0$  is the initial density.

3 The original JH2 model for float glass was developed to simulate its ballistic  
4 performance based on limited experimental testing data. Based on static and  
5 dynamic laboratory test results on architectural annealed glass, together with  
6 previous experimental investigations on window glass, Zhang and Hao derived  
7 material constants for the modified JH2 model [17, 18]. Through comparisons with  
8 experimental results, the JH2 model with newly derived material constants has been  
9 proved to give reliable results in simulation of annealed glass window responses  
10 under shock and impact loads. In the current numerical model, this model is adopted  
11 for annealed glass material.

## 12 *PVB*

13 Experimental investigations on PVB material show that PVB exhibits viscoelastic  
14 property under quasi-static loading. As a polymer material, PVB fails at strain over  
15 200%. The Mooney-Rivlin model is generally chosen to model the hyperelastic  
16 behavior of PVB when it is loaded slowly. However, dynamic tensile tests performed  
17 on PVB by various researchers [10, 22-24] indicated that as loading rate increases  
18 the behavior of PVB gradually transforms from viscoelastic into elasto-plastic like  
19 with an initial rise in stress and then a reduced modulus before failure. At high strain  
20 rate, the behavior of PVB could even be brittle. The authors have recently conducted  
21 some dynamic direct tensile tests on PVB material as well [25]. It was found that  
22 when the strain rate is above  $2s^{-1}$ , PVB basically behaves as an elasto-plastic material.  
23 Under blast loading, the strain rate that a material experiences is generally above  
24  $10s^{-1}$ . A strain rate dependent elasto-plastic material model is therefore chosen for  
25 PVB.

1 The PVB initial Young's modulus, yield stress, and ultimate failure stress at  
2 various strain rates reported by previous researchers are collected and fitted into  
3 equations with respect to strain rates for these quantities as:

$$E_{initial} = 30.591 \left( \frac{\dot{\epsilon}}{\dot{\epsilon}_0} \right)^{0.271} \text{ MPa} \quad (9)$$

$$\sigma_{yield} = 2.167 \left( \frac{\dot{\epsilon}}{\dot{\epsilon}_0} \right)^{0.399} \text{ MPa} \quad (10)$$

$$\sigma_{failure} = 27.689 \left( \frac{\dot{\epsilon}}{\dot{\epsilon}_0} \right)^{0.040} \text{ MPa} \quad (11)$$

4 where  $\dot{\epsilon}$  is the strain rate that material experienced, and  $\dot{\epsilon}_0$  is a reference strain rate  
5 of  $1s^{-1}$ . The density of PVB is  $1100kg/m^3$  and the Poisson's ratio is 0.495. The 2<sup>nd</sup>  
6 modulus measured in the experiments is averaged, and 11MPa is taken in this study.  
7 The fitted stress-strain relations are programmed and implemented into LS-DYNA  
8 code to conduct the numerical simulations. Detailed description of the strain-rate-  
9 dependent elastoplastic model for PVB material is provided in reference [26]. It is  
10 worth noting that dynamic tensile tests show that after PVB specimen fractures,  
11 most of its deformation gradually recovered indicating viscoelastic nature of PVB  
12 material [25]. Since there is not yet testing data on the dynamic unloading behaviour  
13 of PVB, it is difficult to exactly model its unloading response of PVB, and in general  
14 assumptions and simplifications have to be made. Some previous articles [27-29]  
15 have discussed the basis and accuracies of using an elasto-plastic model for plastic  
16 materials similar to PVB. In this study, much attention has been paid to the response  
17 of PVB during numerical verification. Through comparing with field testing results,  
18 the numerical model with the adopted material model for PVB was found to be  
19 capable to properly simulate the response of the laminated glass windows.  
20 Nevertheless, once testing data describing the unloading behaviour of PVB material  
21 is available, a more accurate material model for PVB is to be generated.

## 22 *Steel frame and anchor*

23 A linear elastic material model is used for the steel frame with steel density  
24  $7800kg/m^3$ , Young's modulus 200GPa, and Poisson's ratio 0.3. The choice of a simple  
25 material model rather than a more complicated model for the window frame is  
26 because the designed window frame in the field test is thick enough, and no material

1 yielding or plastic deformation was observed on the steel frame. A simple elastic  
2 material model could help to improve computational efficiency without sacrificing  
3 precision.

4 For the fixture bar and bolt in the interlayer anchorage systems, to model the  
5 possibility of steel yielding, PIECEWISE LINEAR PLASTICITY material model (MAT024)  
6 in LS-DYNA is used. The yield's stress and tangent modulus are set to 270MPa and  
7 470MPa respectively.

#### 8 3.1.4 Contact algorithm

9 To define the interactions between different components of laminated glass  
10 window, the contact options in LS-DYNA is utilized. Different types of contact are  
11 defined based on the specific material and connections that are described below.

12 The current work focuses on studying the influence of window frame to the  
13 overall response of the glass windows. As described above in the field test, the  
14 laminated glass panes are proposed to be clamped by two pieces of steel frames,  
15 which are bolted together firmly. The friction on the contact surfaces between glass  
16 and steel prevents glass pane slipping out of the frame. The contact option  
17 AUTOMATIC SURFACE TO SURFACE in LS-DYNA is used with static friction coefficient  
18 of 0.7 and dynamic friction coefficient of 0.5 to simulate the interaction between  
19 glass and steel frame. The inner window frame is initially fixed. A clamping pressure  
20 of 10MPa is applied to the outer window frame to model the bolt clamping effect.

21 Observation in field blast tests on laminated glass found that delamination hardly  
22 occur between fractured glass and PVB interlayer. Glass debonding from PVB layer is  
23 therefore not modelled. The contacting nodes between PVB and glass are merged  
24 together. This simplification helps to improve computation efficiency.

25 The interactions between PVB interlayer and fixtures bars, as well as PVB with  
26 fixture bolts are modeled with three dimension contact, AUTOMATIC SURFACE TO  
27 SURFACE option, in the numerical simulation. Considering the relatively low modulus  
28 of PVB material as compared to steel, soft constraint formulation instead of the  
29 default penalty formulation is used in LS-DYNA.

## 1 3.2 Calibration

2 To verify the accuracy and reliability of the numerical model, two window tests  
3 are numerically simulated. The laminated pane 1-1-1 with 50mm bite depth  
4 subjected to 10kg TNT at 10m stand-off distance without boundary failure is  
5 simulated first. Then the numerical simulation is extended to model the laminated  
6 pane with total failure along window boundary, i.e., the laminated pane tested by  
7 Hooper et al. [9], which was pulled out of the frame completely and propelled into  
8 the testing cell, is modelled.

### 9 3.2.1 Without boundary failure

10 In test 1, the 7.52mm laminated glass window with 50mm bite was subjected to  
11 the blast loads from 10kg TNT detonated at 10m stand-off distance. The recorded  
12 reflected pressure in the field test is simplified as shown in Figure 12 and applied to  
13 the outer glass ply.

14 Figure 13 shows the comparison of the simulated and field tested laminated glass  
15 windows. As shown, the numerical model manages to simulate the overall response  
16 of the laminated pane under the field blast load. The glass plies are both extensively  
17 shattered with the most severe damage at the corners. The central region of the  
18 laminated pane is relatively intact. There is no sign of the cracked laminated pane  
19 being pulled out of its boundary. Figure 12 compares the deflection histories at the  
20 pane centrals. The predicated central displacement shows good agreement with the  
21 measured data in the field test. A maximum deflection of 268mm is predicated in  
22 comparison with 275mm in the field test. The error is less than 3%, indicating good  
23 numerical predictions. The numerical model slightly underestimates the peak  
24 deflection of the laminated pane, due to a number of uncertainties, especially the  
25 errors in material models.

### 26 3.2.2 With boundary failure

27 Hooper et al. tested a 7.52mm thick laminated glass window of dimension 1.5m  
28 by 1.2m with 25mm bite depth [9]. Under the blast loading from 30kg TNT  
29 equivalent charge detonated at 14m away, the laminated glass pane was totally

1 pulled out of the window frame and was propelled into the testing cell. To further  
2 calibrate the numerical model, this test is also simulated in the study. The measured  
3 reflected pressure reported in [9] is fitted and applied to the laminated glass as  
4 shown in Figure 14.

5 Figure 15 depicts the failure state of the prediction using the numerical model  
6 and that observed in the field blast test. As can be seen, the numerical model  
7 predicts a very similar failure mode of the laminated glass pane. In the numerical  
8 model the glass around the centre of the pane is relatively intact, while the damage  
9 of the pane in the Hooper's test is not visible due to the stochastic speckle pattern  
10 applied on the window surface for digital image correlation. The cracked laminated  
11 panes are both pulled out of the window frame along four sides, leaving only pane  
12 corners held by the frames at the instant shown in Figure 15. Under the blast loading,  
13 the cracked glass pane works as a whole without interlayer rupture and flies into the  
14 room. The pane central deflection histories shown in Figure 14 provides further  
15 evidence that the numerical model agrees well with the measured data in the field  
16 test. The laminated glass pane deforms under the effect of the blast pressure. In the  
17 field test at about 11ms, the shattered laminated pane is completely pulled out of  
18 the window frame with a maximum central deflection of 265mm. In comparison, the  
19 laminated pane in the numerical model is totally pulled out of the window frame at  
20 around 12ms with a central deflection of 275mm. The laminated glass pane without  
21 any constrains from its frame continued to travel into the room.

22 Through the above comparisons, it can be concluded that the numerical model  
23 gives reasonable predictions of the laminated glass window response to blast loads.  
24 The constraining effect of window frame can be properly simulated by the numerical  
25 model.

## 26 3.3 Numerical results and analysis

### 27 3.3.1 Frame bite depth

28 The effect of bite depth on the responses of laminated glass windows is studied  
29 by numerically simulating 7.52mm thick 1.5m x 1.2m laminated panes with four  
30 different bite depths, i.e., 10mm, 20mm, 50mm, and 70mm with bite depth to frame

1 dimension ratios of 0.008, 0.017, 0.042 and 0.058, respectively. Three load cases are  
2 considered in the analysis, i.e., a low level blast with 20kg TNT explosive detonated  
3 30m away to generate Level C blast loading ( $P_r^+=27\text{kPa}$  lasting about 7ms) following  
4 GSA standard [30]. The magnitude of blast pressure is estimated following UFC 3-  
5 340-02 [31]. An intermediate high level blast with reflected pressure recorded in test  
6 1 in the field test above, and a high level blast pressure as recorded in test 2. The  
7 reflected pressures applied are presented in the following with respective pane  
8 deflection histories, where the negative phases are also included.

9 When subjected to the low level blast load, the simulations indicate that all the  
10 laminated panes survive the blast load without joint failure as shown in Figure 16.  
11 Despite glass cracking occurs on all laminated panes, interlayer ruptures are not  
12 found. The pane central displacement histories in Figure 17 show that all the four  
13 laminated panes respond similarly to the blast load. The panes with 10mm and  
14 20mm embedment respond marginally slower than the other two panes with deep  
15 bites. This is probably because the shallow bite depth resulted in slightly flexible  
16 window system, which as a result responds a bit slower. Higher deflections were  
17 found on the panes when they rebounded, which were due to the effect of negative  
18 pressure.

19 As shown in Figure 18, under the intermediate level blast load, glass plies of all  
20 panes experience severe damage, but the laminate panes with different bite depths  
21 respond very differently. With 10mm and 20mm bite depths, the laminated panes  
22 are easily pulled out of their window frames and pushed into the room. When  
23 increasing the bite depth to 50mm and 70mm, the laminated panes are restrained  
24 between the steel frames. Figure 19 shows the time histories of displacement at  
25 pane centrals. As shown, the laminated pane with only 10mm embedment is quickly  
26 pulled out of its frame and propelled under the blast load. With slightly larger  
27 embedment (20mm), the pane receives more restraint from its frame, and responds  
28 slightly slower, but failure along window boundary still occurs. For the two laminated  
29 panes with larger bite depths (50mm and 70mm), they survive the blast load without  
30 being pulled out of their frames. As can be seen from Figure 19, the pane with 50mm  
31 embedment reaches a bit higher maximum deflection (268mm) as compared with

1 the other pane with 70 mm bite depth (251mm). This is because of insufficient  
2 friction restraint from the 50mm bite frame, and relative in-plane sliding still  
3 happens. The 50mm deep bite manages to withstand the pull forces.

4 As shown in Figure 20, under the high blast loading the glass plies of all the  
5 laminated panes are shattered. The two laminated panes with shallow bites (10mm  
6 and 20mm) fail by being pulled out from their frames perimetrally. When increasing  
7 bite depth to 50mm, the pane survives the positive phase blast loading and reaches  
8 a maximum deflection of 316mm without any joint failure (Figure 21). The pane  
9 rebounds, together with the action of negative blast pressure a higher central  
10 deflection is resulted. When the pane central deflection reaches 354mm, the  
11 restraint of the 50mm deep bite is no longer able to hold the laminated pane from  
12 the suction of negative pressure. Pulling-out failure happens to the laminated pane  
13 embedded in 50mm bite frame during rebound at about 40ms. It is worth noting  
14 that the PVB interlayer on the laminated pane with 50mm bite experiences  
15 significant deformation, but no interlayer rupture is found to the PVB membrane  
16 which is because the relative in-plane sliding occurred between the pane and frame  
17 when the pane is deflecting into the room. The relative slide of the pane reduces the  
18 deformation of the PVB interlayer and reliefs its rupture potential. When increasing  
19 the bite depth to 70mm, a more robust fully fixed support is created for the  
20 laminated glass window. The steel frame with deep bite holds the laminated pane  
21 firmly when it is under the action of the blast pressure. No joint failure occurs to the  
22 laminated pane. However, as shown in Figure 20 the interlayer of the laminated  
23 pane is torn when the pane is deflecting inward. This is because of the large  
24 magnitude blast pressure resulting significant shear and flexural deformation to the  
25 laminated pane. The deep-bite frame restrains the pane firmly from any in-plane  
26 sliding. Without any relief from sliding, the PVB interlayer ruptures when its principal  
27 strain reaches the ultimate capacity.

28 Through the above analysis, it can be found that providing sufficient bite depth is  
29 an effective way to mitigate joint failure of laminated windows, reduce the risk of  
30 glass pane being pulled out of its frame under blast loading. However, if the

1 boundary is too rigid, as in the case with 70 mm bite, it might make the glass pane  
2 more vulnerable to blast load.

### 3 3.3.2 Interlayer anchorage

4 Trawinski et al. [13] introduced two types of anchorage measures to reduce the  
5 risk of laminated glass pane being pulled out of its frame. The two anchorage  
6 measures are (1) using fixture bars along two sides of the window frame to hold the  
7 extended PVB interlayer and (2) using fixtures bolts to fix extended PVB strips to the  
8 window frame. The details of these two measures are illustrated in Figure 9c to f.  
9 1.5m × 1.2m doubly laminated glass pane with three glass plies and two PVB  
10 interlayers are modelled in the study. 50mm wide steel frame is assumed to be  
11 installed to clamp the pane in position. 10mm diameter high strength steel rods are  
12 positioned along the two vertical sides of the window frame as the fixture bars,  
13 which are anchored at their both ends with full restraint into the frame. The  
14 extended PVB interlayer wraps around the fixture bar (Figure 9e and f). When the  
15 PVB interlayer is under tension as the laminated pane deforms, the two steel bars  
16 will hold the interlayer to prevent it from sliding. For the laminated pane with fixture  
17 bolt retrofit, an extra 100mm PVB strips are extended from the laminated pane. 20  
18 pieces of M10 high strength steel bolts are fixed perimetally around the window  
19 frame at 200mm spacing. These fixture bolts go through the pre-drilled holes on the  
20 extended PVB strips, and are fully fixed onto the wall (Figure 9c and d). Similar to the  
21 fixture bars, these bolts will hold the PVB interlayer when the laminated pane is  
22 under blast loading. To check the effectiveness of these two interlayer anchorage  
23 measures, the laminated glass pane without any interlayer anchorage retrofit is also  
24 modeled to provide reference. The responses of laminated glass windows are  
25 simulated with four different levels of blast loadings, i.e. a small scale blast with 20kg  
26 TNT detonated at 30m distance to generate blast load following GSA standard Level  
27 C, an intermediate level blast load as in the current field blast Test 1 with 10kg TNT  
28 detonated at 10m distance, a large-scale blast load as in Test 7 with 20kg TNT  
29 detonated at 7.2m distance, and an extra-large-scale blast load with 90kg TNT  
30 detonated at 10m stand-off distance, respectively. The magnitude of blast loads are



1 estimated following UFC 3-340-02 [31]. The reflected pressure time histories are  
2 shown in Figure 22, where the negative phases are also included.

3 Figure 23 depicts the ultimate failure states of laminated glass windows with  
4 fixture bar and fixture bolt, and without any interlayer anchorage retrofit but 50 mm  
5 bite only. As shown, under the minimum level of blast loading (small scale), none of  
6 the laminated panes experiences any noticeable damage which is due to the large  
7 flexural strength of the 21.04mm (6mm glass, 1.52mm PVB, 6mm glass, 1.52mm PVB,  
8 and 6mm glass) doubly laminated pane, as well as the increased inertial resistance  
9 owing to the large mass, as compared to the 7.52 mm laminated glass window  
10 discussed above. Figure 22a shows the central displacement histories. As shown,  
11 barely any difference can be found on the central displacement histories among the  
12 three laminated panes. When the laminated glass windows are under intermediate-  
13 scale blast loading, glass cracks can be observed on the laminated panes (Figure 23).  
14 The central displacement histories indicate a maximum deflection (about 42mm) is  
15 reached on the laminated pane without anchorage measure. Due to the extra  
16 restraint effects from the fixture bars and fixture bolts, lower central deflections are  
17 found on the two corresponding laminated panes with interlayer anchors (37mm  
18 and 35mm respectively). Fixture bolts appears to provide slightly better resistance  
19 perimetally to the cracked laminated pane with a bit smaller central deflection  
20 resulted. Under the large-scale blast loading, the laminated pane without any  
21 interlayer anchorage is pulled out of its frame along the two vertical boundaries. A  
22 maximum central deflection of about 245mm is predicted. But with the friction  
23 resistance from the top and bottom boundaries and the four corners, the cracked  
24 pane finally comes to a rest within the window frame. The negative blast pressure  
25 appears to have insignificant influence on the doubly laminated panes that it does  
26 not suck out the glass pane. This is probably because of the heavier mass of the  
27 doubly laminated panes comparing with the 7.52mm singly laminated panes. In  
28 comparison, the fixture bar is quite effective that they successfully hold the  
29 laminated pane along its two vertical boundaries from being pulled out of the  
30 window frame. The maximum pane central deflection is about 195mm. The fixture  
31 bolts provides similar anchorage effect to the sliding interlayer. As a result of bolt

1 anchor, a maximum central deflection of about 193mm is predicted, which is 21%  
2 lower than the case without boundary anchorage. Comparing the effectiveness of  
3 fixture bar and fixture bolt, it seems that the bolt anchors yield slightly better  
4 performance of laminated glass windows under the current blast loading. This is  
5 because the bolt anchor provides additional resistance to stop the interlayer from  
6 sliding perimetally. In comparison, the fixture bar can only hold the interlayer along  
7 its two sides. As shown in Figure 23, because the bars are fixed into the frame on  
8 their two ends, the 1.5m long 10mm diameter steel bars yield under the substantial  
9 pulling forces from the PVB interlayer when the laminated glass pane is under large  
10 blast loads. The deformation of the fixture bars curves into the window which makes  
11 the laminated pane slide inward and consequentially leads to slightly higher central  
12 deflection. Close observation on the extended PVB strips at the fixture bolts; it can  
13 also found that interlayer tearing could be a major potential problem when the  
14 laminated pane is under substantial tensile forces from the blast load. Under the  
15 extra-large-scale blast loading from 90kg TNT detonated at 10m stand-off distance,  
16 the laminated pane without interlayer anchorage is directly pushed into the testing  
17 room with laminated pane being widely pulled out of the window frame around the  
18 four sides. The laminated pane with fixture bars does not survive the substantial  
19 blast load either. The fixture bars pull the laminated pane back from sliding initially.  
20 However, under the large blast pressure, the laminated pane experiences substantial  
21 deformation, which pulls the fixture bars and causes significant bending and  
22 curvature on the steel bars. Rupture eventually occurs on the PVB interlayer near  
23 and in contact with the anchor bars. After PVB rupture along the anchor bars, the  
24 laminated pane is pulled out of the window frame from the top and bottom sides  
25 and then pushed into the room. Similar response is observed on the laminated pane  
26 with fixture bolts. As shown in Figure 23, as the laminated pane deforms, the  
27 extended PVB strips are torn through the bolt holes. From the deflection histories  
28 shown in Figure 22d, it can be observed that the central deflections of all the three  
29 laminated panes kept increasing, indicating they are flying into the room. The fixture  
30 bar and fixture bolts delay marginally the failure along the window frame.

1 Through the above analysis, it can be found that anchor bars and bolt can help to  
2 mitigate the potential of joint failure along laminated glass pane boundaries. Their  
3 effectiveness is quite obvious when the laminated glass windows are subjected to  
4 certain levels of blast loadings. When the window is under low level blast loading,  
5 the laminated pane itself and the clamping window frame could provide sufficient  
6 resistance to stop pane from being pulled out. The effect of interlayer anchor is not  
7 apparent. When the laminated glass window is subjected to large blast loading, PVB  
8 interlayer rupture at the locations in contact with the anchor bars and PVB interlayer  
9 tearing at the fixture bolts might occur, resulting in the failure of the anchoring  
10 system.

#### 11 **4 Sliding boundary**

12 Zhang et al. [6] studied the effect of releasing boundary restraint on improving  
13 the anti-blast performance of laminated glass windows. Comparing with the fully  
14 fixed boundary condition, the capacity of laminated glass panes were found to  
15 increase when using pinned boundary which allowed the rotation of the laminated  
16 pane along its boundary. It was found that using a flexible window boundary might  
17 mitigate the damage of the laminated pane under blast pressure. Following this idea,  
18 a sliding boundary is proposed, which allows the laminated pane to slide freely in the  
19 direction of blast pressure. The laminated pane slides backward to mitigate the blast  
20 load when the air blast wave pushes it inward. A doubly laminated pane with three  
21 glass plies laminated by two PVB interlayers with sliding boundary is tested in this  
22 study to examine the effectiveness of using this flexible boundary to mitigate  
23 window glass damage. To minimize the possibility of the glass pane failure or rupture  
24 immediately after the application of blast loading, strong doubly laminated glass  
25 panes as described above were tested to observe the influences of sliding boundary.

26 In the field blast test 7 described above, two doubly laminated glass windows (as  
27 shown in Figure 2b), one with fixed boundary and another one with the proposed  
28 sliding boundary, were tested. The pressure transducer recorded a peak reflected  
29 pressure of 514kPa lasting about 3ms. Figure 24 shows the snapshots of high-speed  
30 camera images for the laminated pane with sliding boundary. It can be observed that

1 after the blast wave arrived at the window 8ms after detonation, the glass plies  
2 cracked immediately. The entire pane also began to slide inward under the action of  
3 the blast wave. At around 20ms, the laminated pane touches the inner window  
4 frame after sliding 50mm inward. The maximum deformation occurred at the pane  
5 central at about 40ms after which the pane starts to rebound.

6 Figure 25 shows the failure patterns of the two laminated panes with fully fixed  
7 and with sliding boundaries in test 7. As shown in Figure 25a, the laminated pane  
8 with sliding boundary suffers severe damage to its glass plies by the substantial blast  
9 load. However, only small joint failure could be observed along its boundary, and no  
10 PVB rupture was observed. The action of blast pressure was vastly mitigated as the  
11 laminated pane slid backwards. In comparison, the laminated pane which was fully  
12 clamped between the steel window frames suffered significant joint failure  
13 especially along its two vertical boundaries (Figure 25b), as well as PVB rupture. The  
14 image of pane failure indicates the fully fixed glass pane with 50 mm bite depth to  
15 the laminated pane is insufficient to prevent pulling-out failure. Comparing the  
16 damage level of the two identical glass panes with different boundary conditions  
17 clearly demonstrates that using flexible boundary can mitigate damages of laminate  
18 glass windows under blast loadings.

19 No LVDT was installed in this test to prevent damaging them by the failed glass  
20 window flying into the testing cell because of the expected large blast loads. Instead  
21 the high-speed camera images are post-processed using a Matlab tracking algorithm  
22 with the aid of the tracking dot matrix on the glass pane to derive the pane  
23 displacement histories. Figure 25c shows the predicted pane failure. As shown, the  
24 numerical model manages to replicate the pulling-out failure along two boundaries.  
25 From the numerical model, pane displacements at various locations can be easily  
26 tracked. The displacement histories at the centres of the two tested panes obtained  
27 by both numerical simulation and high-speed camera images in the field test are  
28 presented in Figure 26. As can be seen, the deflection at pane centre for the pane  
29 with fixed boundary increased immediately under the blast load. The central  
30 deflection rises quickly to a maximum of about 247mm and then recovers as pane  
31 rebounded. The deflection near the window boundary increases simultaneously with

1 pane central. But because of the restraint from window frame, low magnitude of  
2 deflection is resulted. High-speed camera images found the displacement at pane  
3 centre of the laminated pane with sliding boundary increased slower at the  
4 beginning. The displacement is mainly associated with the sliding of the entire pane.  
5 Similar displacement was recorded at the pane boundary until it approximated  
6 50mm, which is the design sliding distance for the laminated pane. The displacement  
7 at pane centre is slightly larger than that at the boundary because of pane  
8 deformation under the blast wave effect. After this instance when the laminated  
9 pane touches the inner window frame, the displacement near the boundary begins  
10 to increase slowly similar to the case with fixed boundary. The central displacement  
11 starts to rise quickly until a maximum deflection is reached (about 251mm) and then  
12 reduces as pane rebounded. Comparing the central displacement histories of the  
13 laminated panes with sliding and fixed boundaries, it can be found the sliding pane  
14 responds slower due to its flexibility. If the sliding distance is deducted from the  
15 deflection at the central, a much smaller central deflection is resulted (about  
16 201mm). It indicates 19% less net maximum central deflection is achieved with the  
17 sliding boundary comparing with the fully fixed boundary, which consequentially  
18 reduces the rupture possibility of the laminated pane. As shown in Figure 25a and b,  
19 no PVB tearing was found on the laminated pane with sliding boundary. However,  
20 some insignificant interlayer rupture was found after closely examining the  
21 laminated pane clamped with fully fixed boundary.

22 Through the above comparisons, it can be found that the sliding boundary can  
23 help to improve the blast resistance capacity of the laminated glass windows. Lower  
24 net central deflection can be achieved by the sliding boundary comparing with the  
25 traditional fully fixed boundary. The field blasting test also shows that the sliding  
26 boundary reduces the potential of joint failure.

## 27 **5 Conclusion**

28 The responses of laminated glass windows were examined through full-scale field  
29 blast tests and numerical simulations. The failure pattern was found to be primarily  
30 joint failure, where the cracked laminated panes were pulled out of the steel frames.

1 Previous field blast test results were collected together with the current testing data  
2 to analyze the formation and influencing factors of joint failure. Numerical model of  
3 laminated glass windows was built and calibrated with testing data, and then used to  
4 study the influence of bite depth to joint failure. Numerical simulations were further  
5 carried out to investigate the effectiveness of two types of interlayer anchorage  
6 systems, namely fixture bar and fixture bolt. The efficiencies of the two anchorage  
7 systems were studied. It was found for intermediate to large scale blast loadings,  
8 interlayer anchorage with fixture bar and fixture bolts can effectively mitigate the  
9 laminated pane joint failure. However, increasing the boundary anchorage increases  
10 the PVB rupture potential. Based on previous founding on boundary effect to the  
11 performance of laminated glass windows, a new sliding boundary was introduced  
12 and tested experimentally. The advantages and disadvantages of the sliding  
13 boundary was discussed and checked through comparison with fully fixed boundary.  
14 It was found the new sliding boundary can effectively reduce the laminated pane  
15 response against blast loading, and also reduce the joint failure possibility.

## 16 **Acknowledgement**

17 The authors would like to thank Australian Research Council for financial support.  
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19 of Technology with its collaborative research scheme under project number KFJJ11-3  
20 is also acknowledged. The first author would also like to thank the University of  
21 Western Australia for providing Ad Hoc scholarship.

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28



- 1 Table 1 Summary of blast test configurations
- 2 **Table 2 Summary of recorded blast loads and estimations using K-B equation**
- 3
- 4

Test No.	Pane No.	Glass thickness (mm)	PVB thickness (mm)	Size (mm × mm)	Boundary condition	TNT weight (kg)	Stand-off distance (m)	Measurement
1	1-1-1	3	1.52	1500 × 1200	Fixed	10	10	Pressure, LVDT, HS
2	2-1-1	3	2.28	1500 × 1200	Fixed	10	9	Pressure, LVDT, HS
3	3-1-1	3	1.52	1500 × 1200	Fixed	10	12.3	Pressure, LVDT, HS
4	4-1-1	3	1.52	1500 × 1200	Fixed	10	9.5	Pressure, HS
6	6-1-1	3	1.52	1500 × 1200	Fixed	20	11	Pressure, HS
7	7-1-1	6	1.52	1500 × 1200	Fixed	20	7.2	Pressure
7	7-1-2	6	1.52	1500 × 1200	Sliding	20	7.2	Pressure, HS

Note: HS indicates high-speed image measurement available; LVDT indicates displacement measurement available; Pressure indicates reflected pressure measurement available.

1

[Table 1 Summary of blast test configurations](#)

2

Test No.	TNT Weight (kg)	Stand-off distance (m)	Field test				K-B equation	
			Positive phase		Negative phase		Positive phase	
			Pr (kPa)	Ir (kPa ms)	Pr (kPa)	Ir (kPa ms)	Pr (kPa)	Ir (kPa ms)
1	10	10	121.1	395.0	28.4	319.7	117.2	293.86
2	10	9	168.6	476.1	35.8	543.5	147.1	330.7
3	10	12.3	82.2	413.3	17.5	261.7	78.1	233.7
4	10	9.5	147.5	436.3	29.6	441.3	130.7	311.2
6	20	11	172.1	534.5	31.8	548.5	157.5	431.2
7	20	7.2	514.3	797.1	43.0	614.3	463.0	703.4

1

Table 2 Summary of recorded blast loads and estimations using K-B equation

2

- 1 Figure 1 Testing site plan
- 2 Figure 2 Illustration of window specimens
- 3 Figure 3 Illustration of window frames
- 4 Figure 4 Recorded reflected pressure and pane central displacement histories
- 5 Figure 5 Snapshots of high-speed images for Pane 4-1-1 and 1-1-1
- 6 Figure 6 Failure patterns
- 7 Figure 7 Comparison of influence of bite depth
- 8 Figure 8 Maximum pane deflections versus bite depth at different reflected impulses
- 9 Figure 9 Laminated glass models
- 10 Figure 10 Mesh size sensitivity test
- 11 Figure 11 Johnson Holmquist Ceramic material model for annealed glass [18]
- 12 Figure 12 Pressure and central deflection time histories in field test and numerical
- 13 simulation for Pane 1-1-1
- 14 Figure 13 Comparison of failure patterns for pane 1-1-1
- 15 Figure 14 Pressure and central deflection histories in field test and numerical
- 16 simulation of Hooper et al.'s test [9]
- 17 Figure 15 Comparison of pane failure patterns of Hooper et al.'s test [9]
- 18 Figure 16 Failure patterns of windows with different bite depths under low level
- 19 blast loading
- 20 Figure 17 Blast load and pane central displacement histories of windows with
- 21 different bite depths under low level blast loading
- 22 Figure 18 Failure patterns of windows with different bite depths under intermediate
- 23 high level blast loading
- 24 Figure 19 Blast load and pane central displacement histories of windows with
- 25 different bite depths under intermediate high level blast loading
- 26 Figure 20 Failure patterns of windows with different bite depths under high blast
- 27 loading
- 28 Figure 21 Blast load and pane central displacement histories of windows with
- 29 different bite depths under high blast loading
- 30 Figure 22 Blast loads and pane central displacement histories for laminated glass
- 31 windows with different anchorage measures
- 32 Figure 23 Ultimate states of laminated glass windows with different retrofits

- 1 Figure 24 Snapshots of high-speed images of the laminated pane with sliding
- 2 boundary
- 3 Figure 25 Failure patterns of the laminated panes in test 7
- 4 Figure 26 Comparison of displacement histories of test 7
- 5
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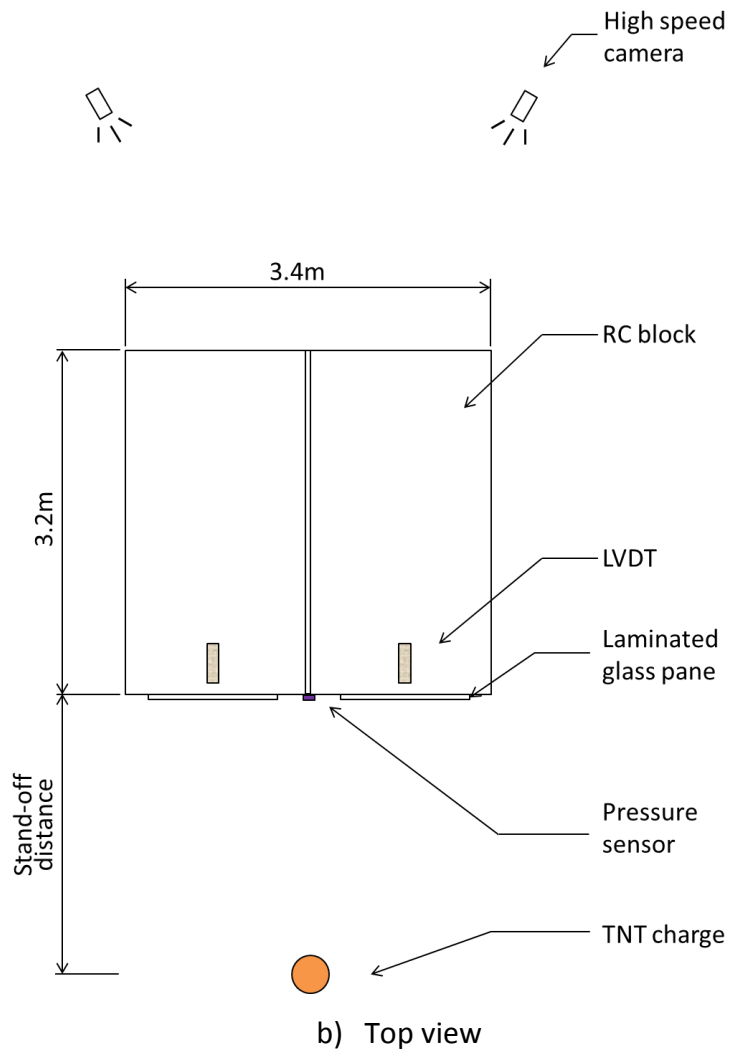
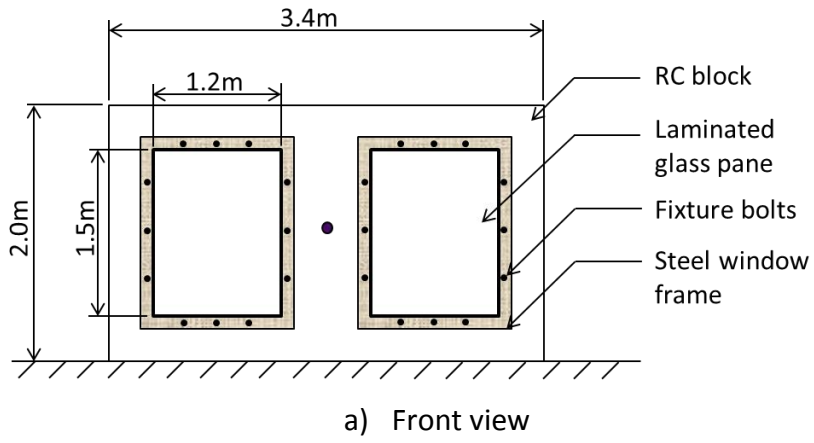


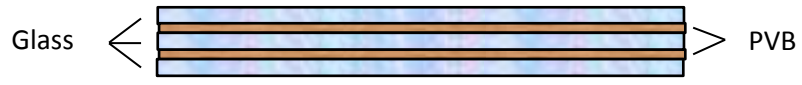
Figure 1 Testing site plan

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a) Singly laminated glass pane



b) doubly laminated glass pane

Figure 2 Illustration of window specimens

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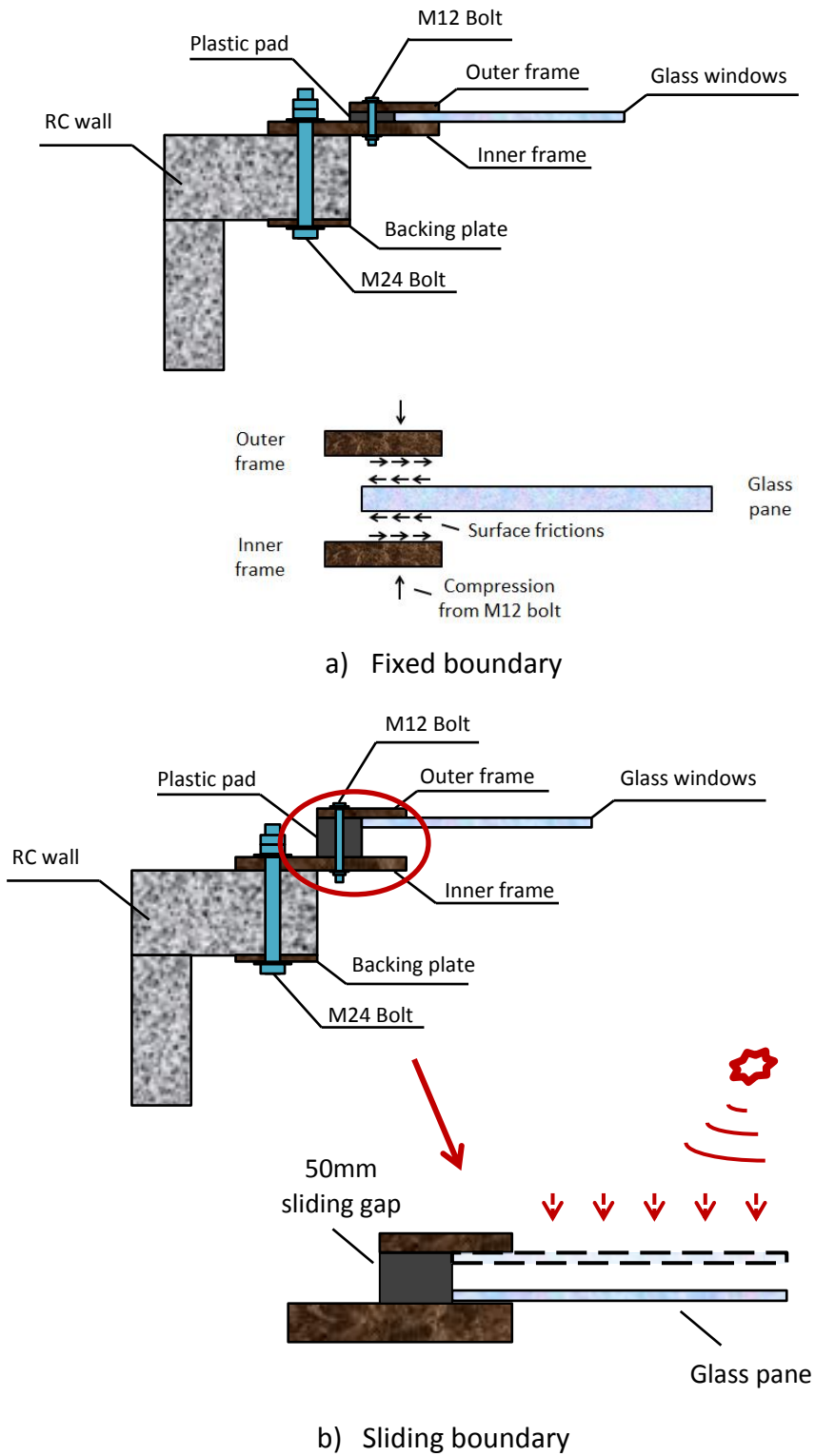
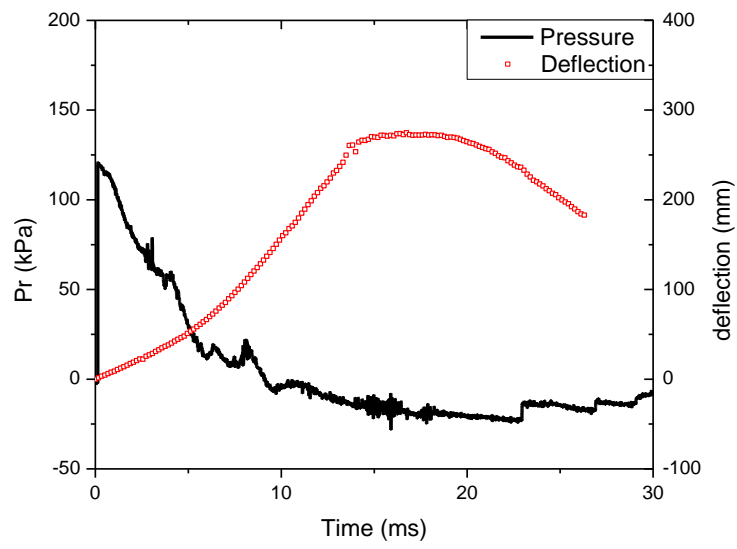


Figure 3 Illustration of window frames

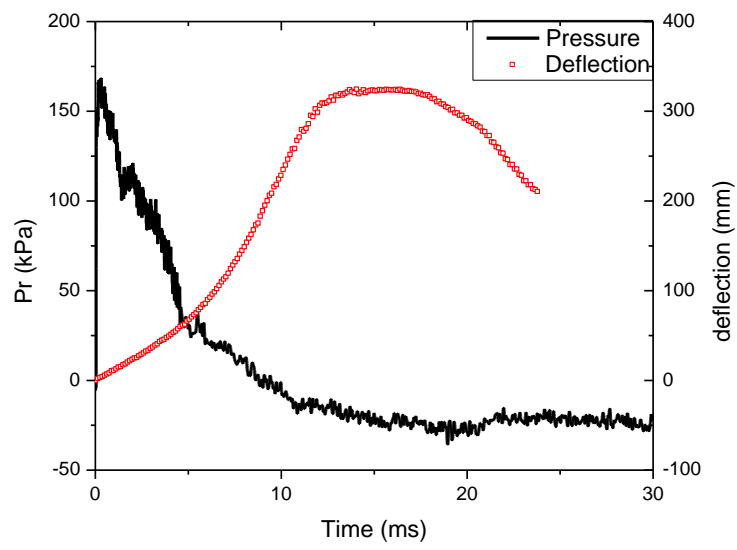
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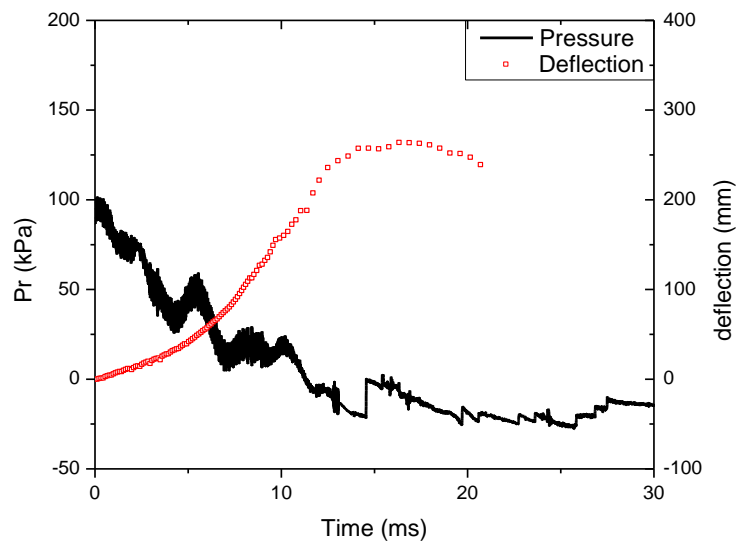




a) Test 1



b) Test 2

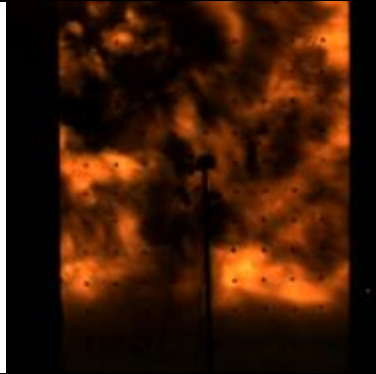
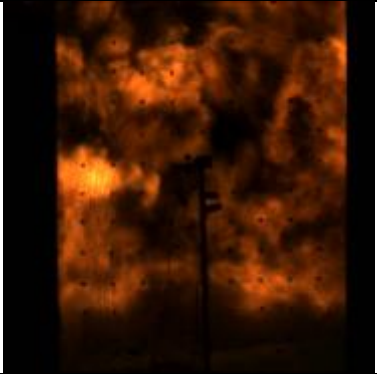






c) Test 3

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Figure 4 Recorded reflected pressure and pane central displacement histories

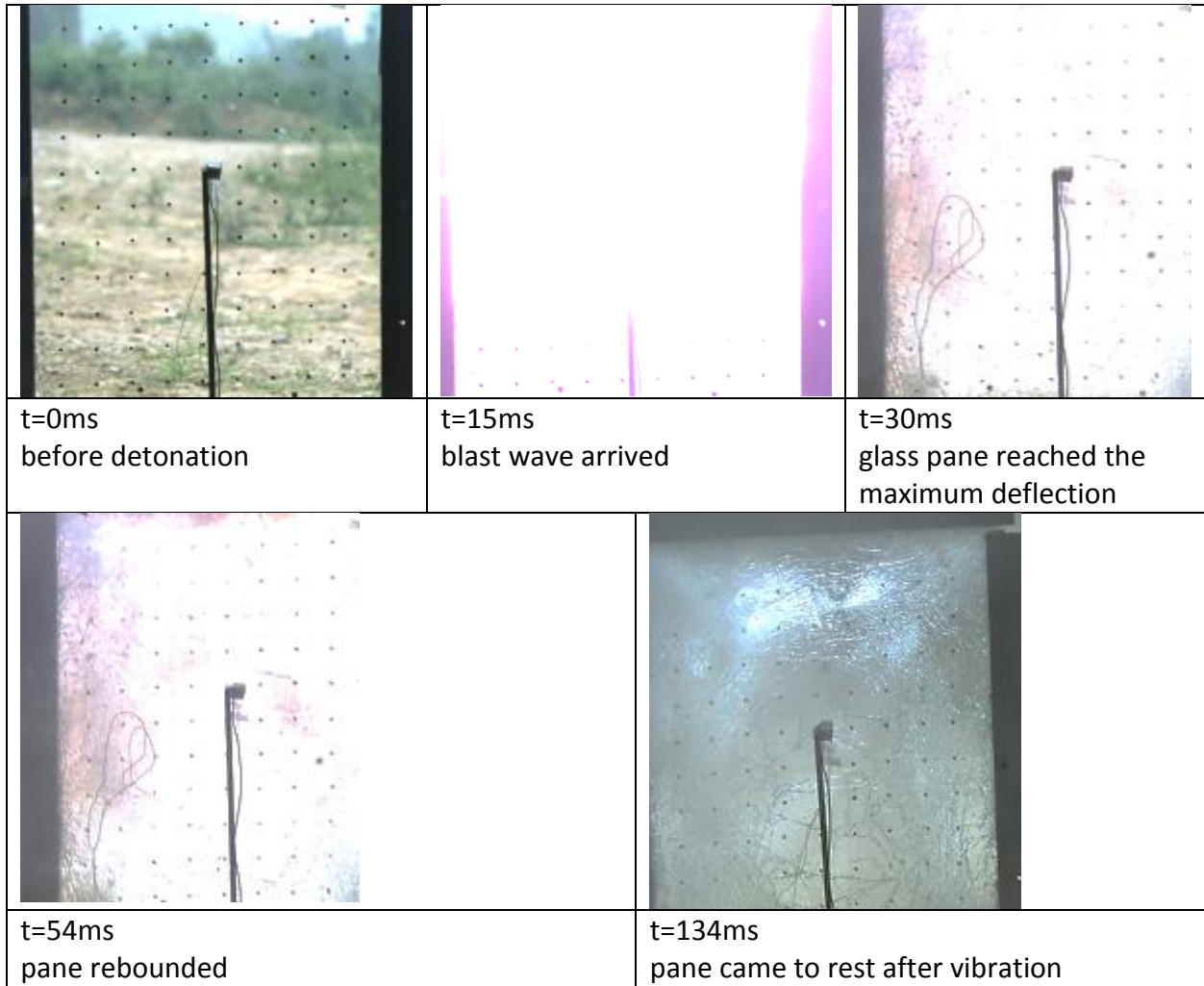
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t=15ms blast wave arrived	t=17ms back glass ply cracked	t=25ms glass pane reached the maximum deflection
		
t=30ms pane rebounded	t=35ms pane was pulled out of the boundary	t=55ms pane was totally pulled out of the frame

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a) Pane 4-1-1

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b) Pane 1-1-1

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Figure 5 Snapshots of high-speed images for Pane 4-1-1 and 1-1-1

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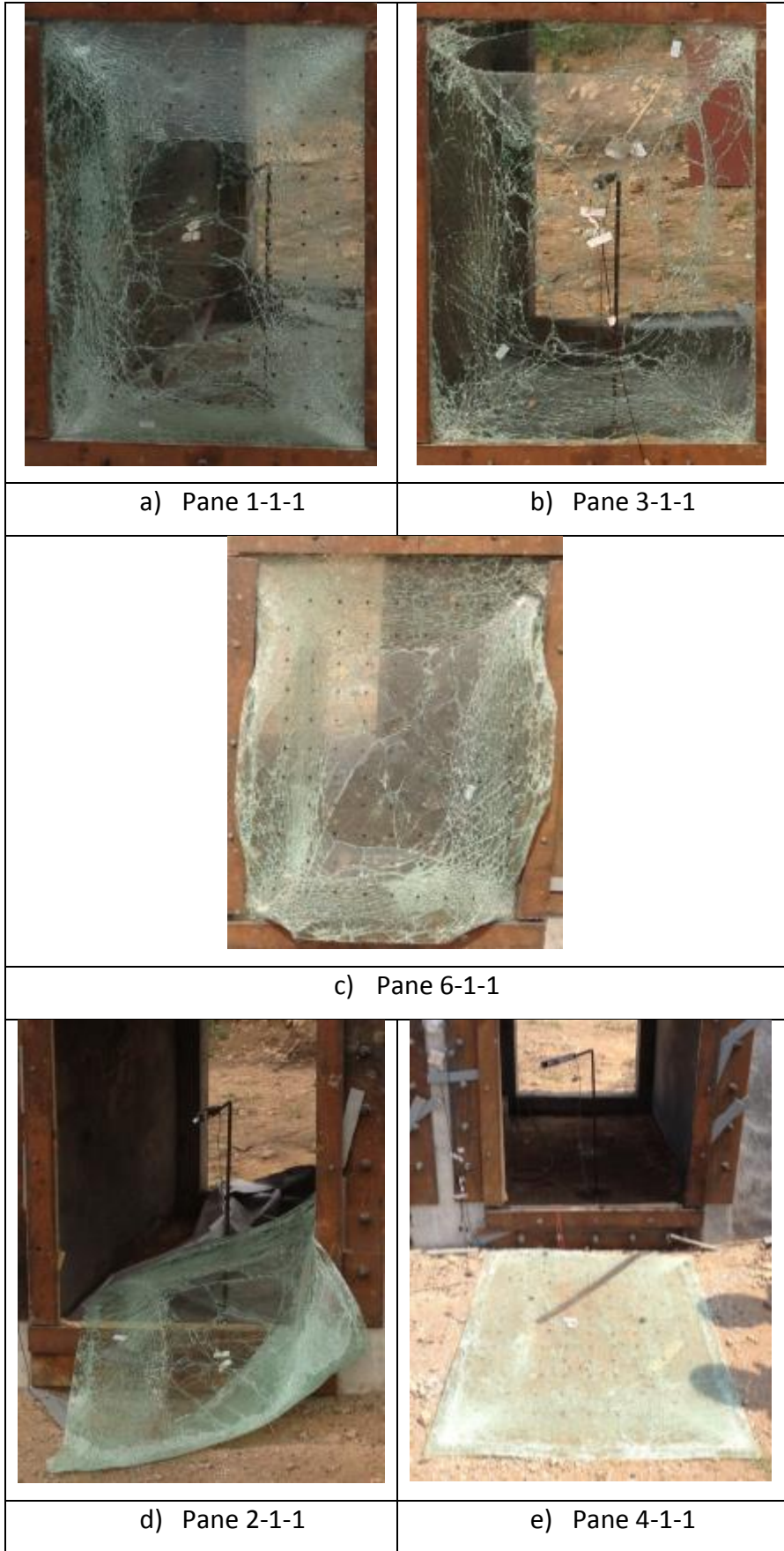


Figure 6 Failure patterns

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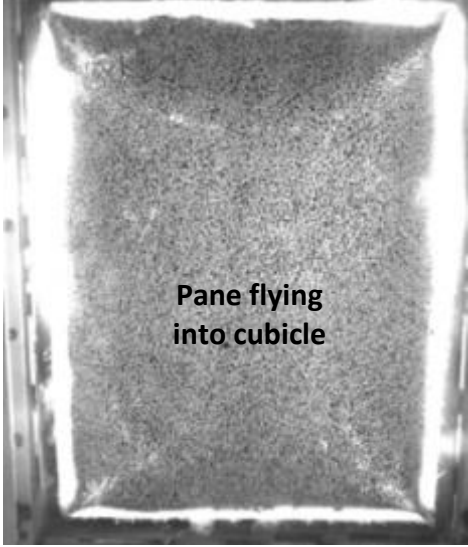
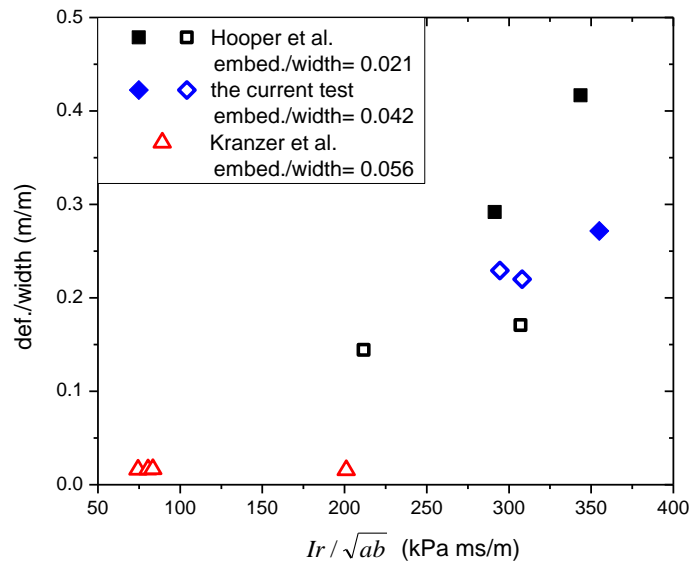
 <p style="text-align: center;"><b>Pane leaving frame during rebound</b></p>	 <p style="text-align: center;"><b>Pane flying into cubicle</b></p>
<p>a) 50mm bite in Pane 2-1-1 (<math>Pr^+=169\text{kPa}</math>, <math>Ir^+=476\text{kPa}\cdot\text{ms}</math>)</p>	<p>b) 25mm bite by Hooper et al. [9] (<math>Pr^+=152\text{kPa}</math>, <math>Ir^+=461\text{kPa}\cdot\text{ms}</math>)</p>

Figure 7 Comparison of influence of bite depth

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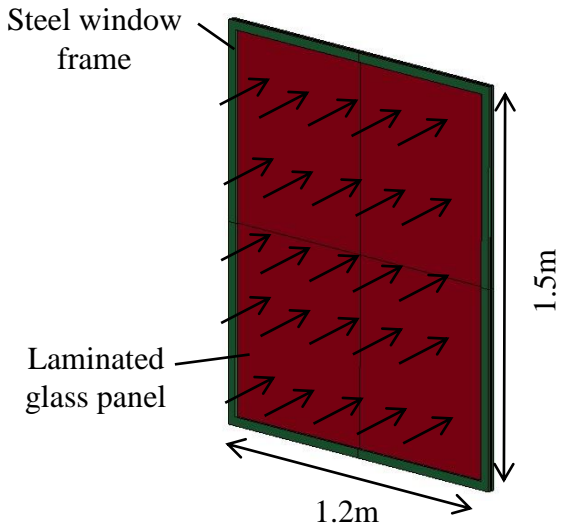
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2 Note: a and b stand for window length and width;  $I_r$  is the reflected impulse.

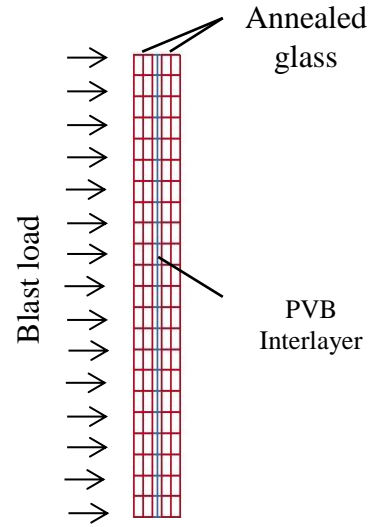
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Figure 8 Maximum pane deflections versus bite depth at different reflected impulses

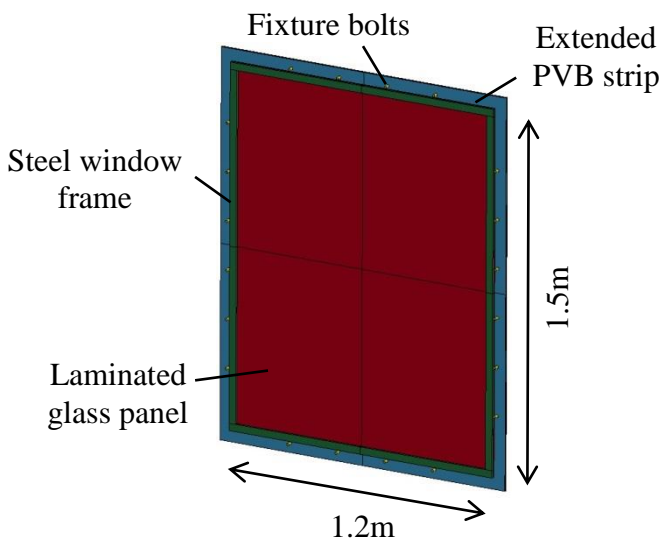
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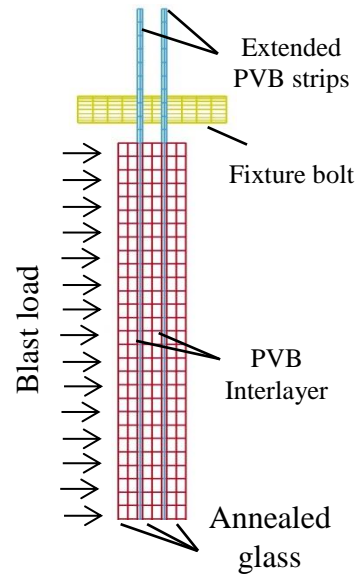
a) Framed glass window without retrofit



b) Element across the thickness direction

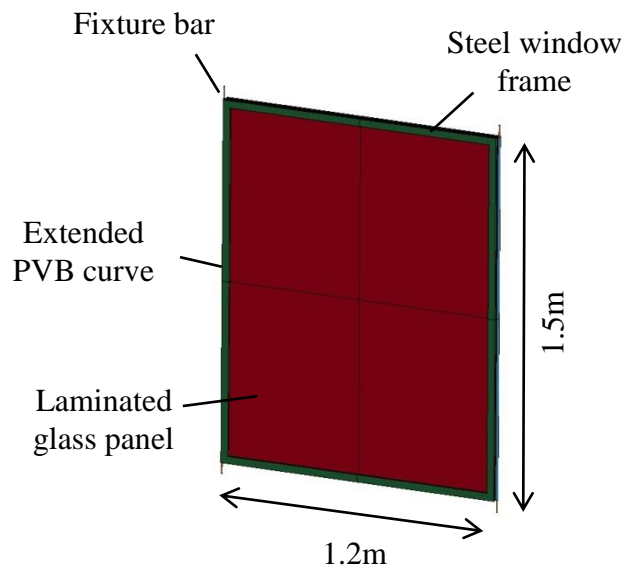


c) Glass window with PVB bolted along boundaries

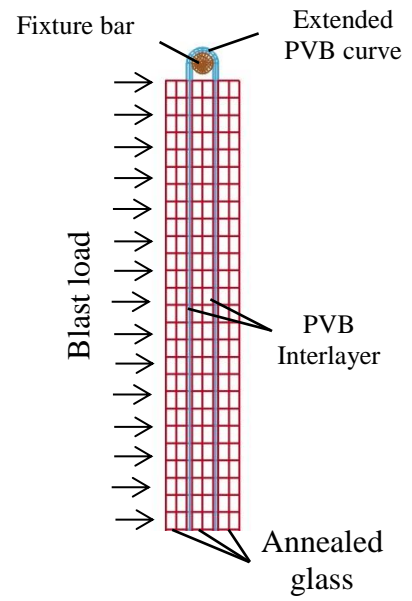


d) Element across the thickness direction





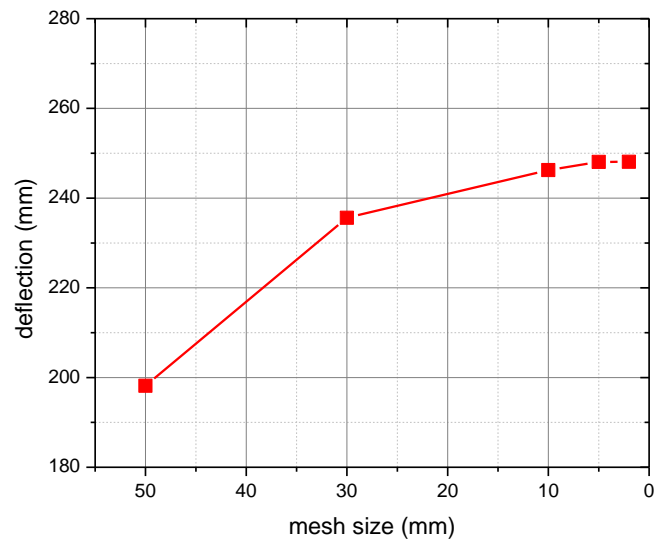
e) Glass window with PVB holded by fixture bars



f) Element across the thickness direction

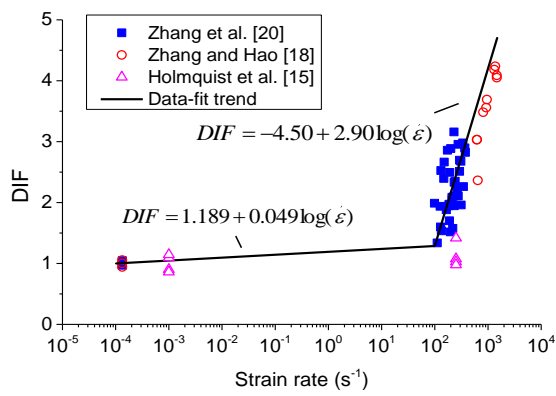
Figure 9 Laminated glass models

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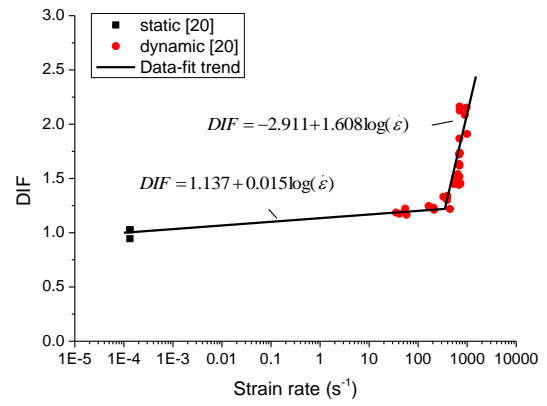


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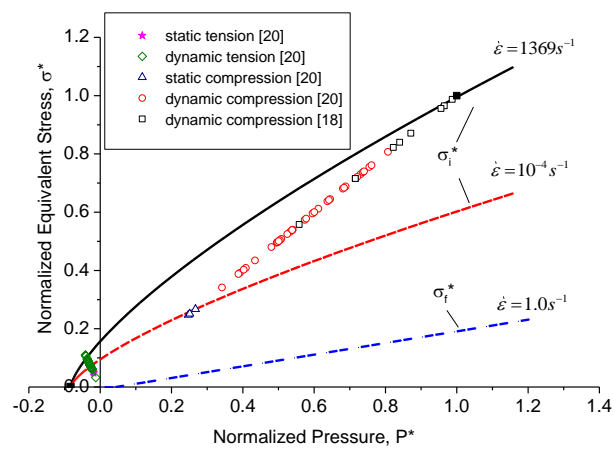
Figure 10 Mesh size sensitivity test



a) Compressive DIFs vs strain rates



b) Tensile DIFs vs strain rates

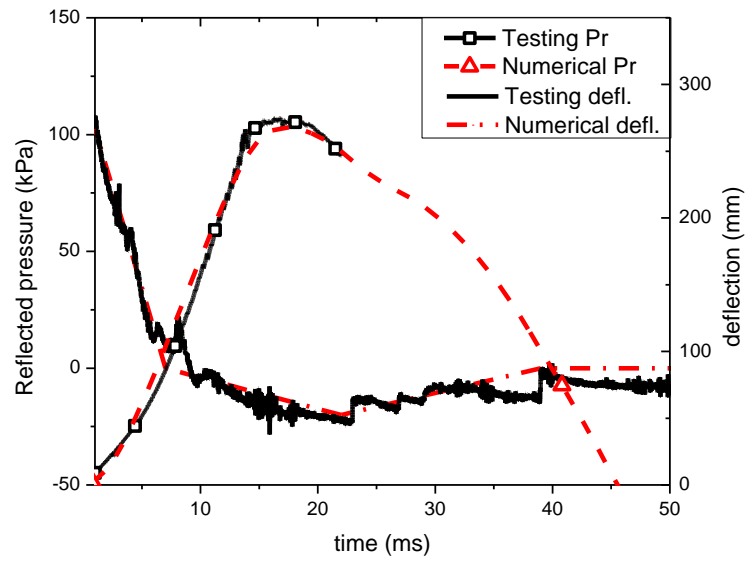


c) Strength model

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Figure 11 Johnson Holmquist Ceramic material model for annealed glass [18]

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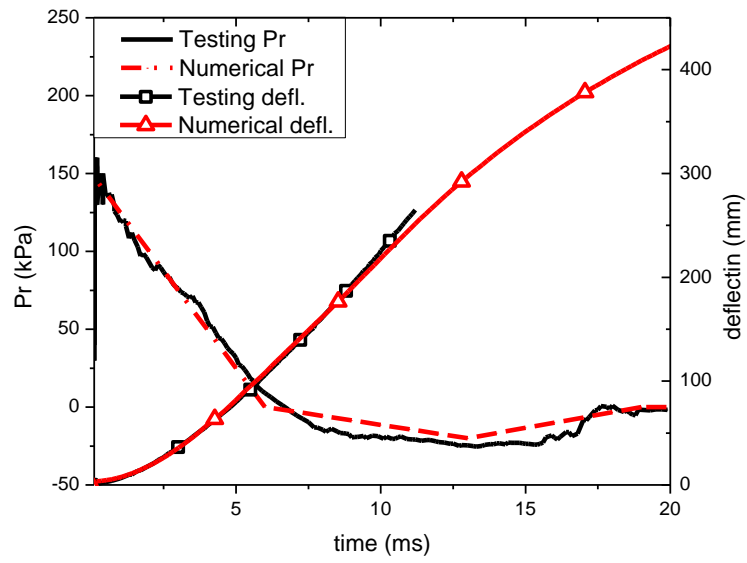
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Figure 12 Pressure and central deflection time histories in field test and numerical simulation for Pane 1-1-1



Figure 13 Comparison of failure patterns for pane 1-1-1

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Figure 14 Pressure and central deflection histories in field test and numerical simulation of Hooper et al.'s test [9]

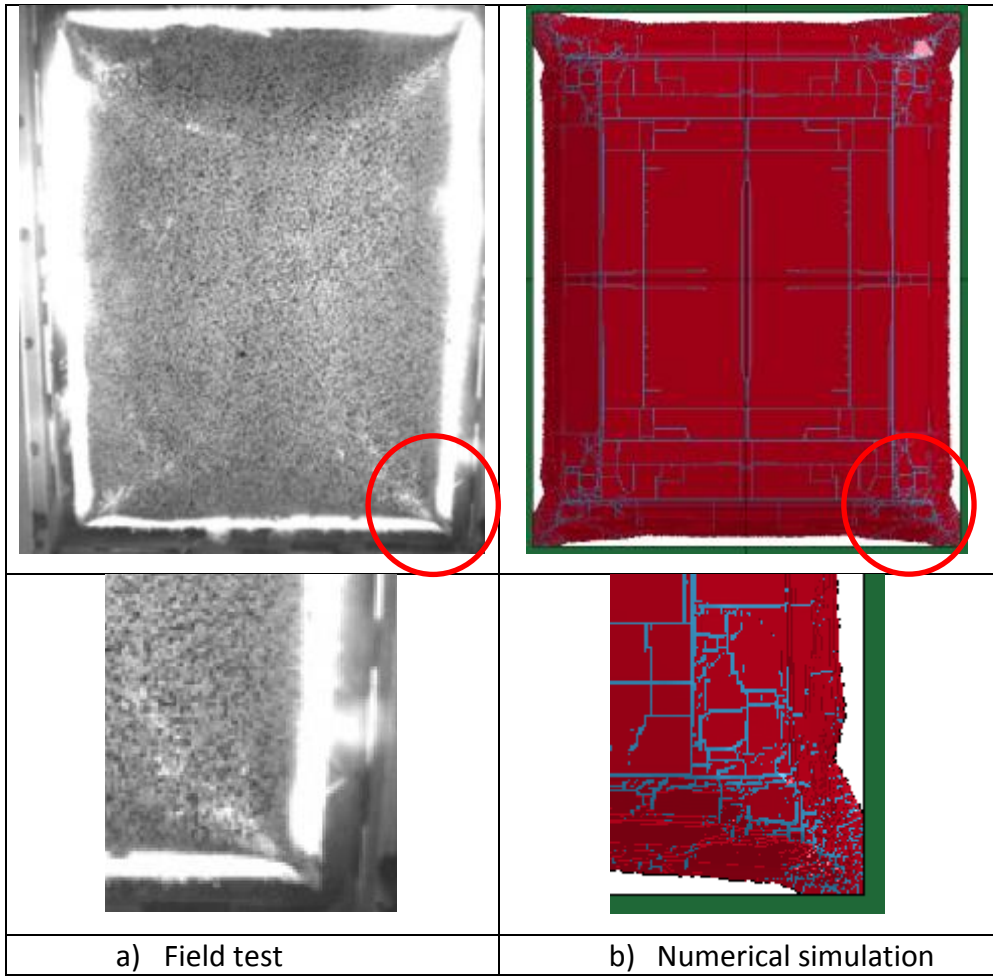
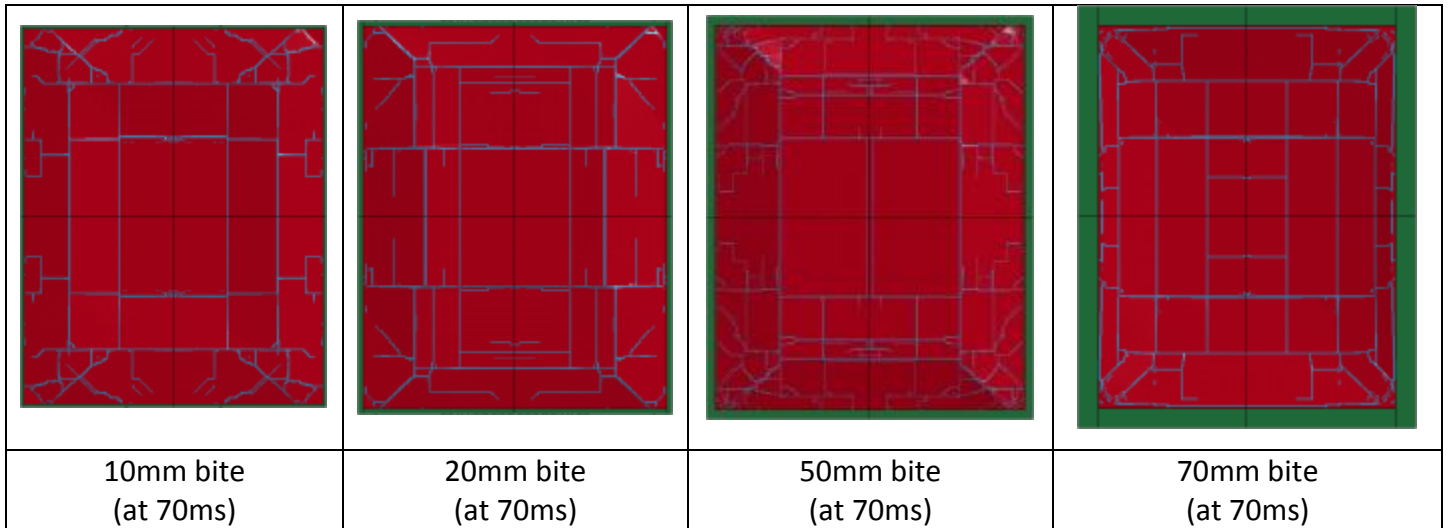


Figure 15 Comparison of pane failure patterns of Hooper et al.'s test [9]

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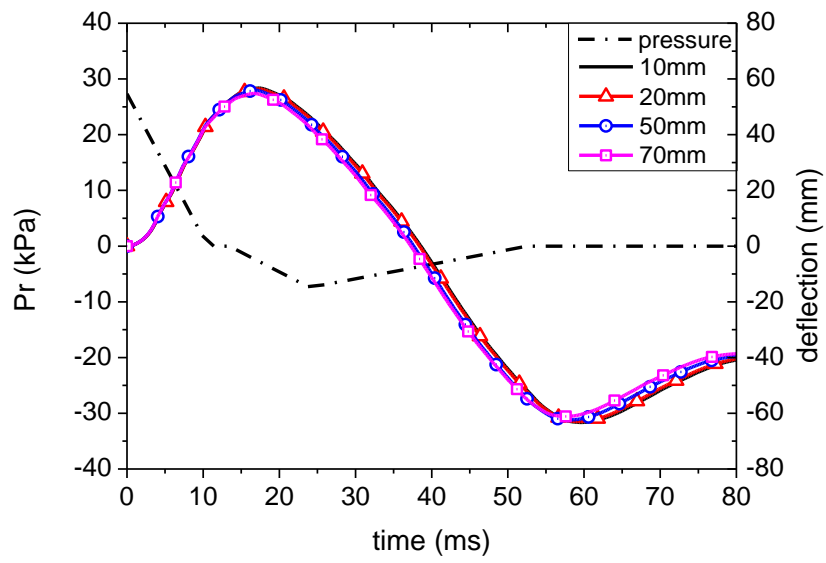


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Figure 16 Failure patterns of windows with different bite depths under low level blast loading

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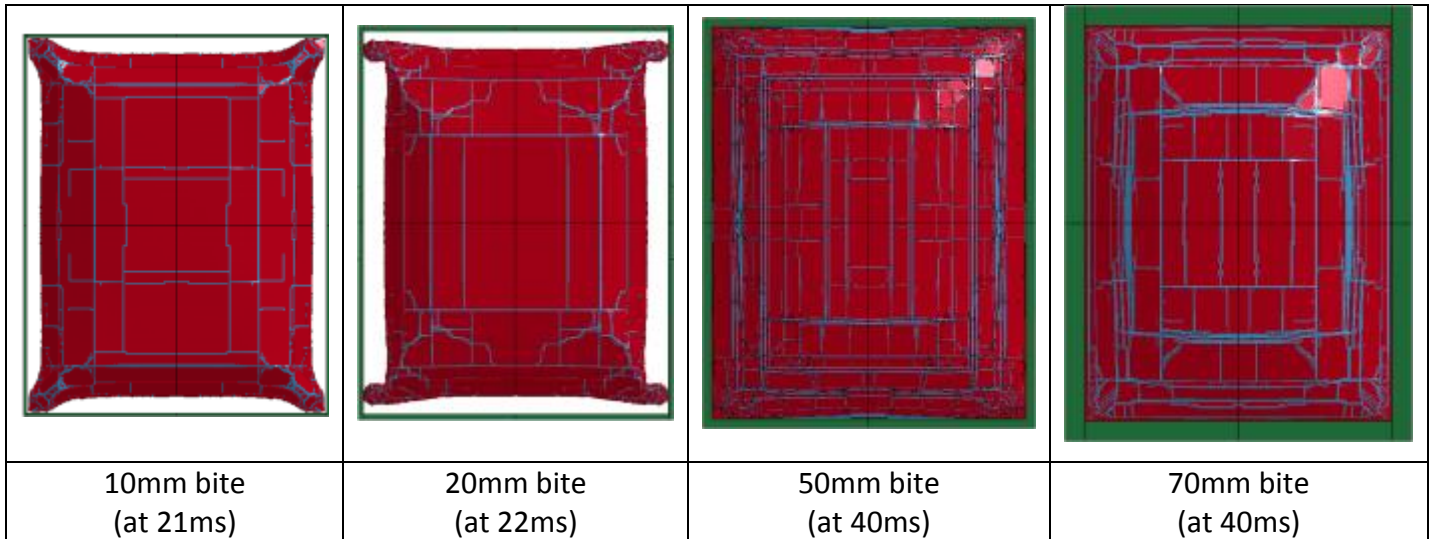
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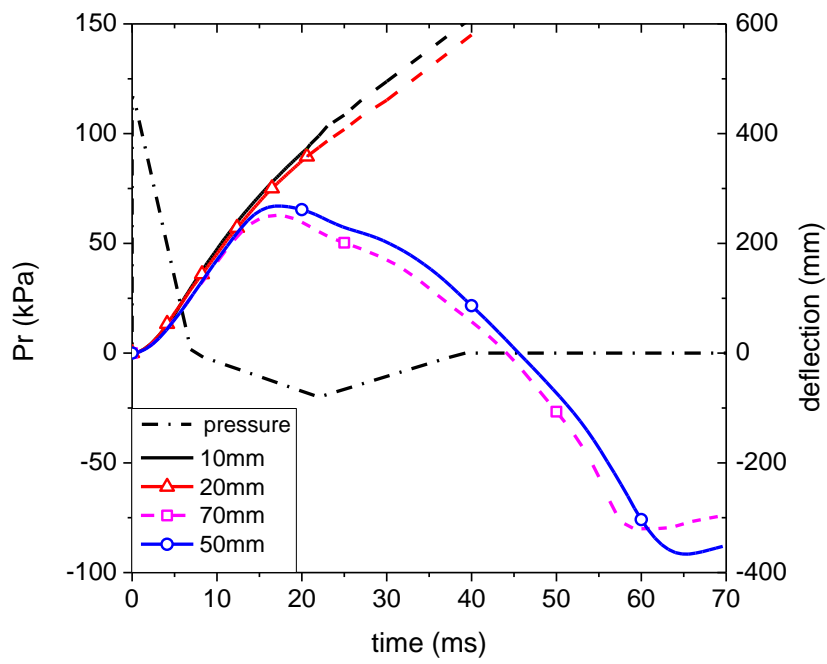
Figure 17 Blast load and pane central displacement histories of windows with different bite depths under low level blast loading

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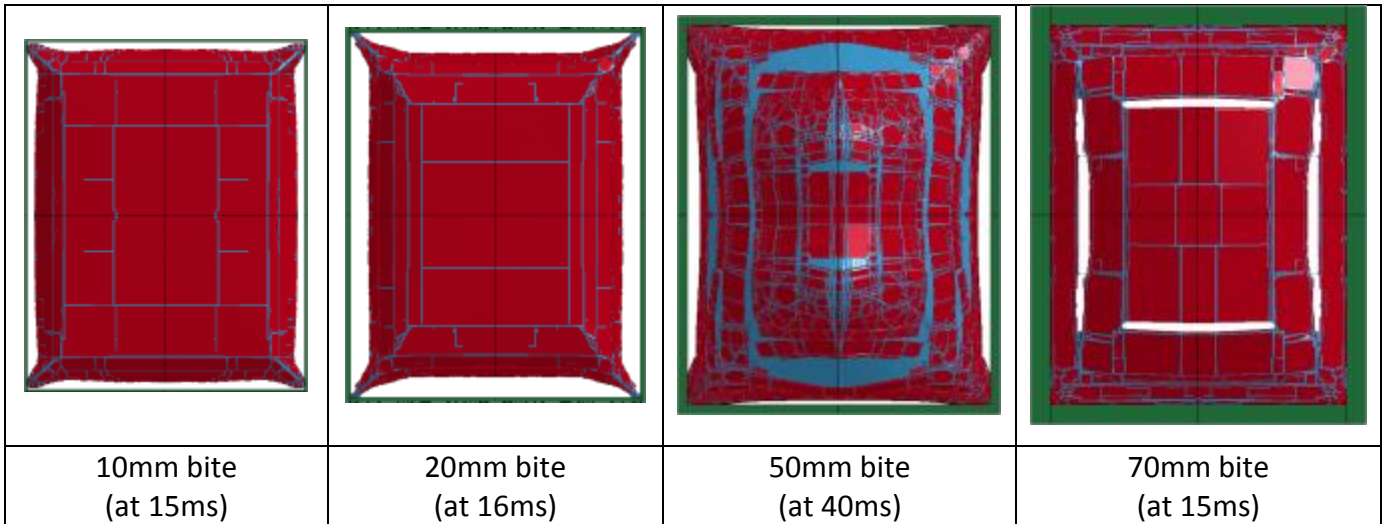
1 [Figure 18 Failure patterns of windows with different bite depths under intermediate high level blast loading](#)

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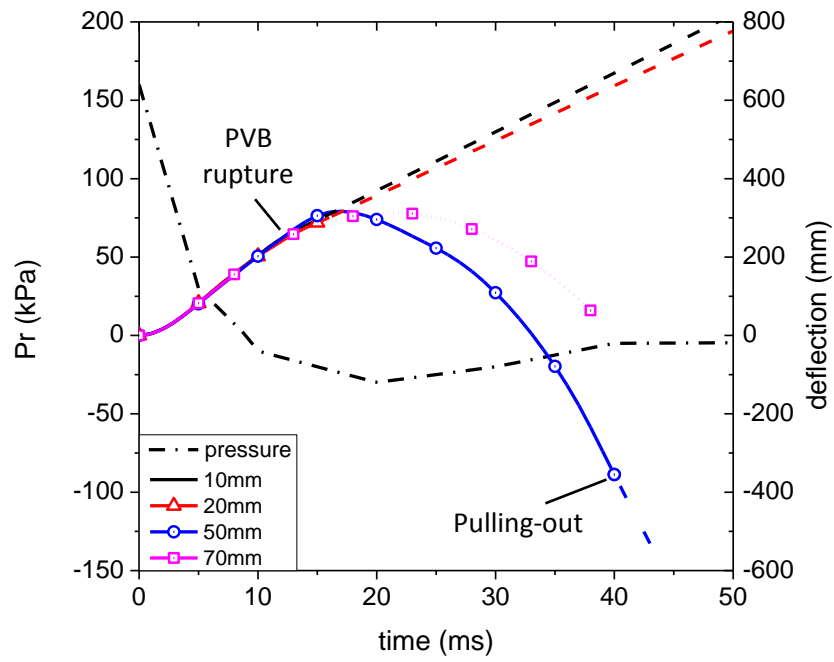
Figure 19 Blast load and pane central displacement histories of windows with different bite depths under intermediate high level blast loading



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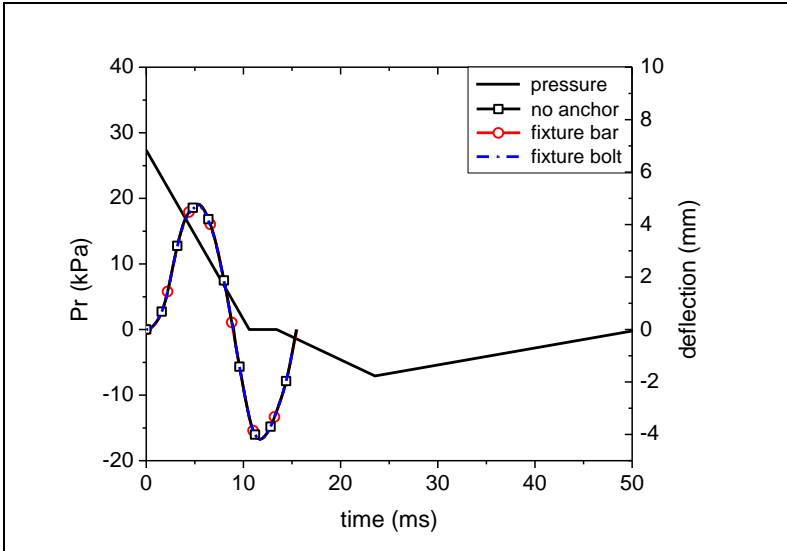
Figure 20 Failure patterns of windows with different bite depths under high blast loading

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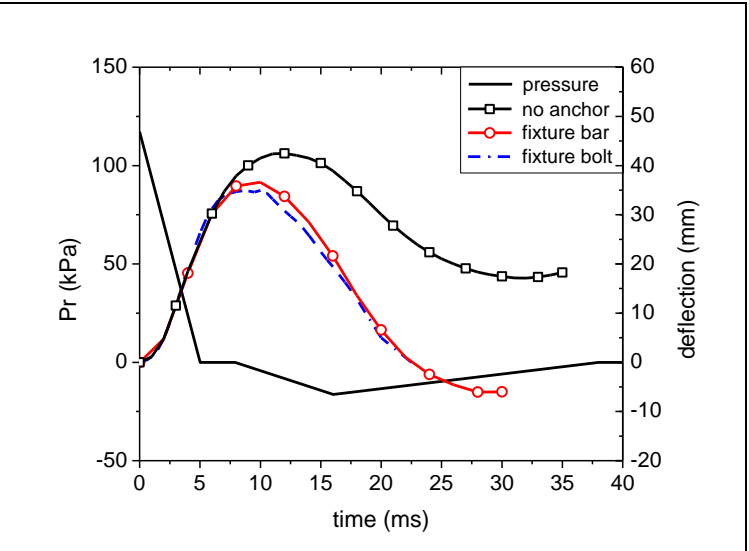


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3  
4

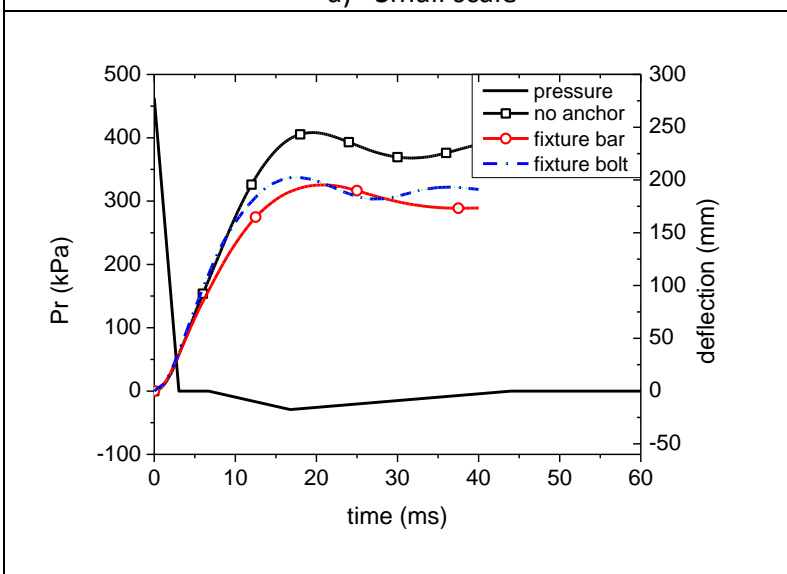
Figure 21 Blast load and pane central displacement histories of windows with different bite depths under high blast loading



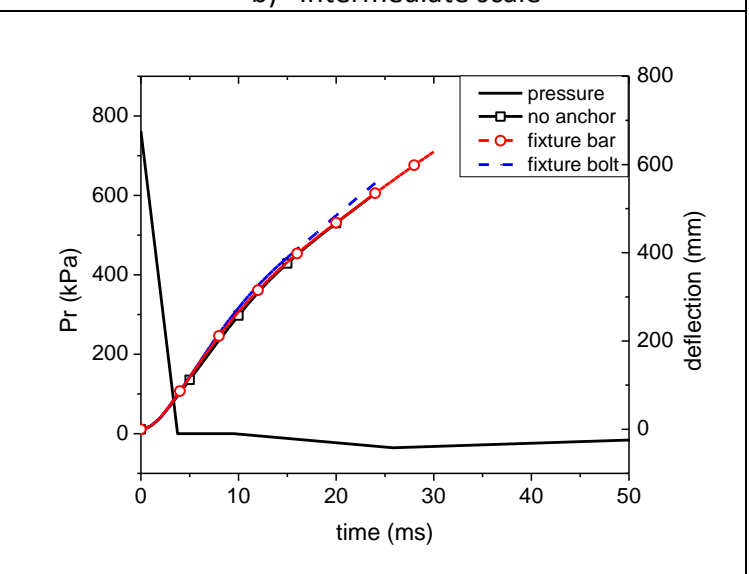
a) Small scale



b) Intermediate scale



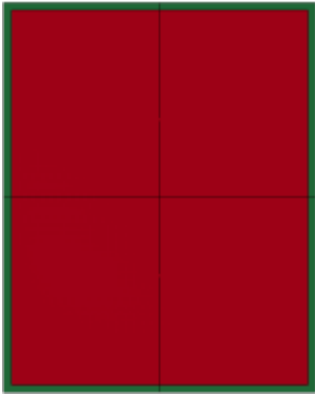
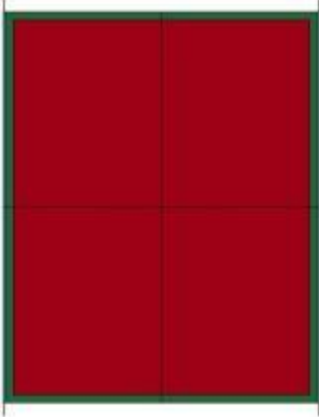
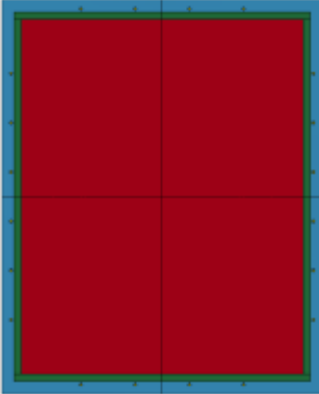
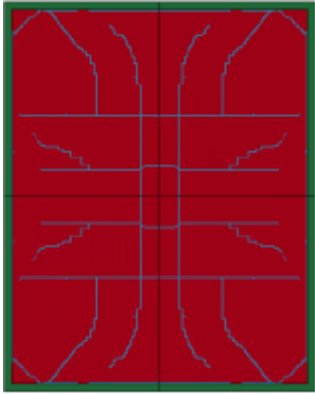
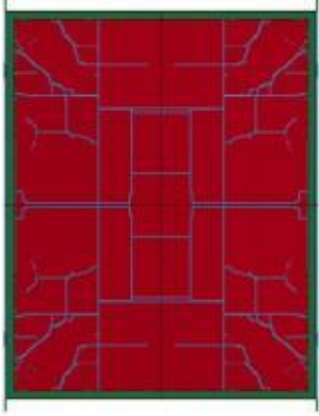
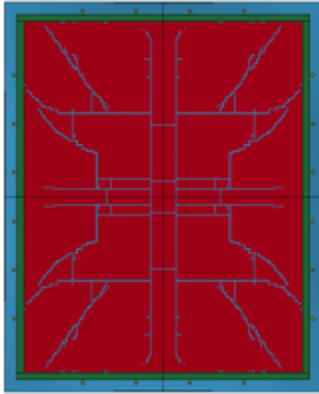
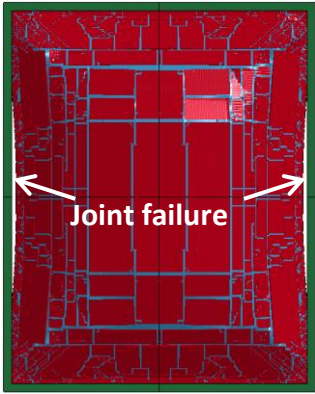


c) Large scale

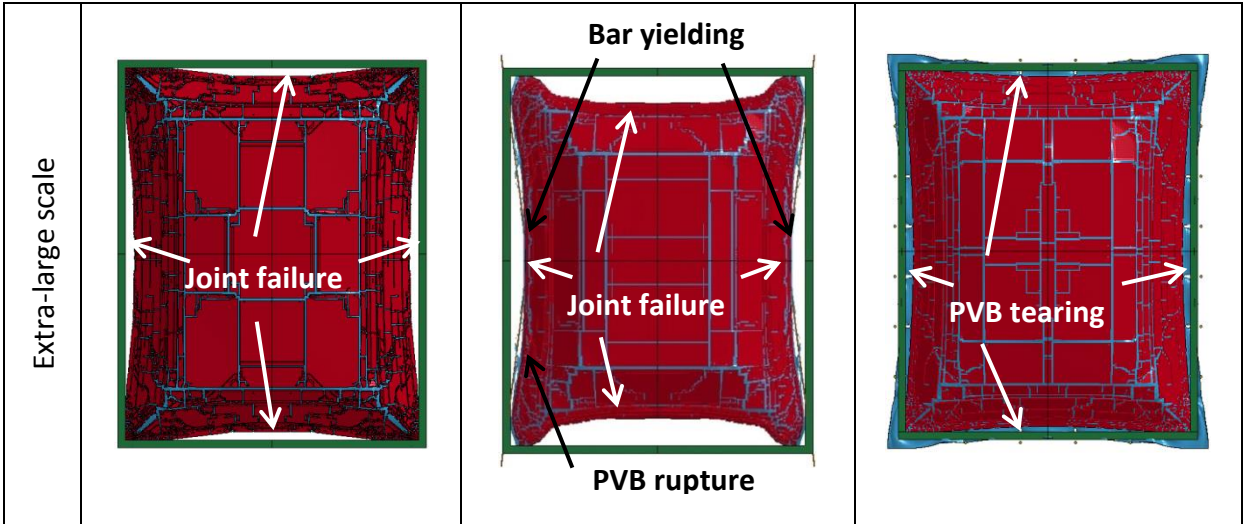


d) Extra-large scale

Figure 22 Blast loads and pane central displacement histories for laminated glass windows with different anchorage measures

1  
2  
3

	No retrofit	Fixture bar	Fixture bolt
Small scale			
Medium scale			
Large scale			

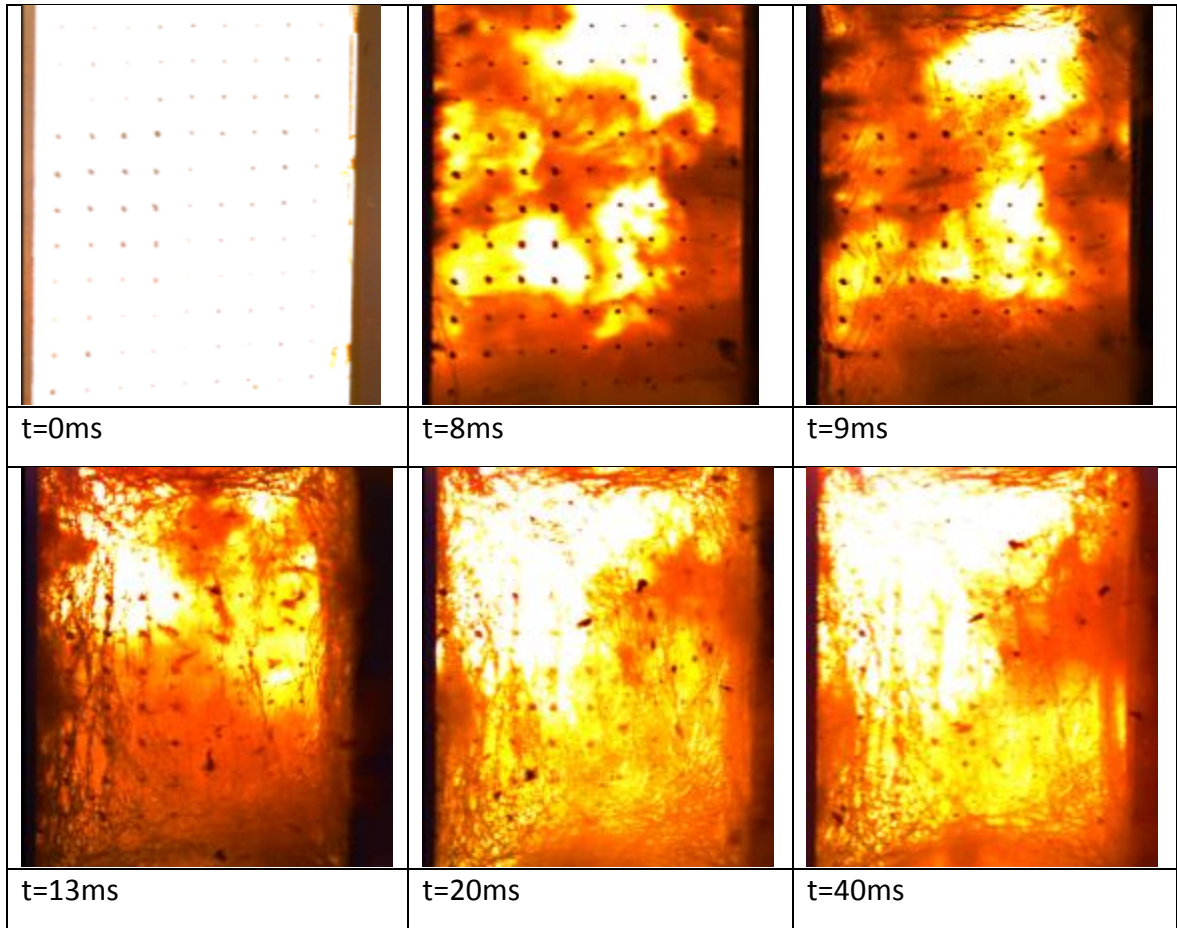


1

Figure 23 Ultimate states of laminated glass windows with different retrofits

2





1

Figure 24 Snapshots of high-speed images of the laminated pane with sliding boundary

2

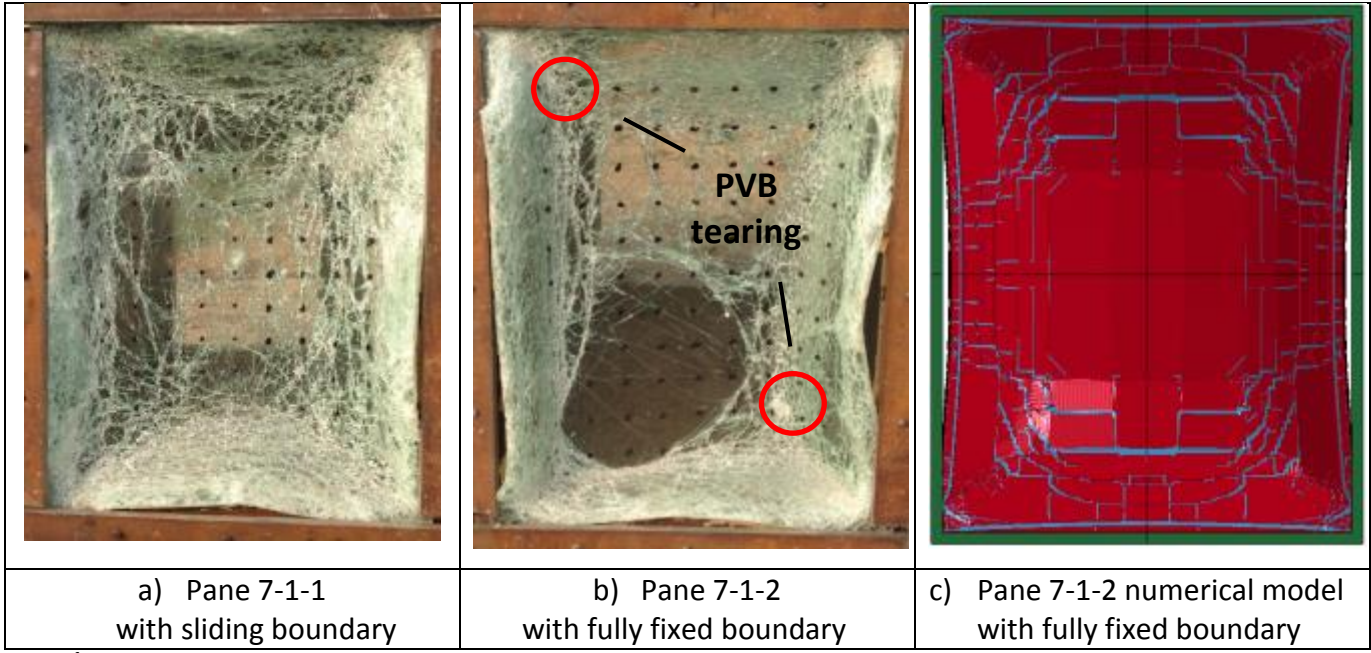


Figure 25 Failure patterns of the laminated panes in test 7

1

2

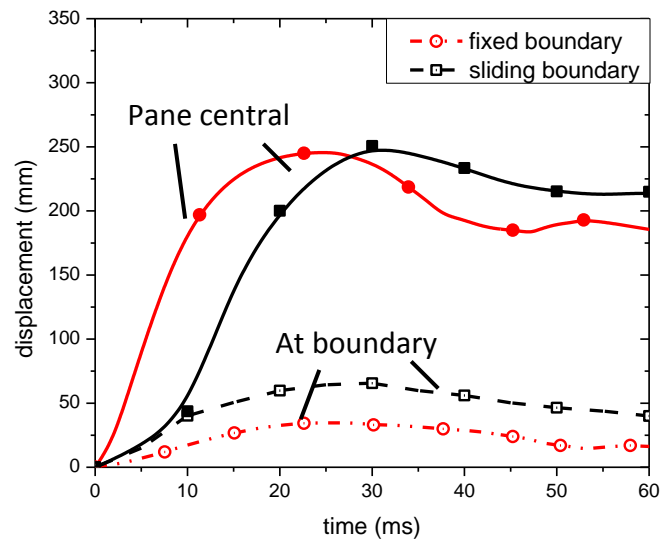


Figure 26 Comparison of displacement histories of test 7

1  
2  
3