The Effect of Cement–Sodium Silicate Grout Compounds on Void Ratio and the Coefficient of Secondary Compression of Treated Fibrous Peat

Reference

ABSTRACT
Peats have low shear strength and high deformation characteristics. Peat is a naturally occurring material that is extremely soft with a high moisture content and exists in an unconsolidated state. The conventional binders used are cementitious materials, and the introduction of a new binder, sodium silicate, with other additives gives a better output than the conventional peat treatment. This article describes a laboratory study on the effect of different compounds of cement–sodium silicate grout on void ratio and the coefficient of secondary compression of fibrous peat. It is shown that by increasing the amount of sodium silicate (within 2.5 %), cement, and kaolinite in treated peat, we were able to decrease the mentioned properties of treated peat.

Keywords
fibrous peat, coefficient of secondary compression, void ratio, cement, sodium silicate, kaolinite

Introduction
The stabilization of highly organic soils such as peat is much more demanding than the stabilization of clay. The traditionally used cement, lime, and lime–cement mixtures are not necessarily the most suitable for these soils. Instead, mixtures of cement and granulated blast furnace slag are often used. These new binders have been extensively researched and tested both in the laboratory and in the field [1–3]. In addition, in most countries, the need for the consolidation of soft soil
deposits and the use of different improvement materials or techniques is extensively high [4,5]. One of the problems with applying this technology to organic soils is that organic matter inhibits cementitious reactions. In fact, for all soils, inorganic and organic, the properties of the stabilized product are extremely difficult to predict, in part because of the lack of understanding of the reactions of the soil, water, and binding agent [6]. Therefore, construction on peat soils has proven to be a challenging task, as this soil is highly compressible, has a low strength, and retains neither its form nor its strength after oxidation.

Huge numbers of poor foundation conditions are reported in all areas of Malaysia where clay, mud, and peat deposits appear. Accordingly, intensive geotechnical research work on soil-improvement techniques in Malaysia is performed at three universities: Universiti Putra Malaysia [7–10], Universiti Teknologi Malaysia [11–13], and Universiti of Malaya [14–16]. Peat or peaty soil normally having a high organic matter content is generally associated with high compressibility and a high magnitude and rate of creep [17]. It may also be associated with poor strength characteristics and a risk of large deformation [5,18]. Thus, peat soil is often viewed as a problematic soil because its engineering properties are inferior to those of other soft soils. It poses serious problems in the construction industry because of its long-term consolidation settlement even when subjected to a moderate load [7,19].

Anderssonl [1] investigated hydraulic cement-based binders for the mass stabilization of organic soils. He describes mass stabilization as a new, environmentally friendly soil improvement method in which stabilizer is mixed into peat, mud, or soft clay. The results have been encouraging, showing that it is possible to provide environmentally friendly solutions and stabilize organic soils cost-effectively. Edil and Fox [20] studied a field test of thermal precompression on peat. They presented a new concept for the improvement of soft ground using moderate heating to control post-construction settlement. Accordingly, they provided the long-term creep behavior of a peat based on laboratory oedometer tests involving step-stress and step-temperature changes. Finally, the feasibility of thermal precompression as a method of controlling or accelerating in situ settlement has been discussed. Hampton and Edil [6] investigated the strength gain of organic ground with cement-type binders. They presented a synthesis of mixture tests conducted in Delft, The Netherlands, and Madison, WI, on several peats and an organic clay showing that the current experimental techniques are not sufficient to create a comprehensive model of strength gain in stabilized organic soil. Research is needed on the fundamental chemical reactions contributing to changes in the geotechnical properties of stabilized organic ground [6]. DenHamer et al. [21] investigated the stabilization of peat by silica-based solidification. They developed a novel soil stabilization concept (called peat silicification) in which soil properties are modified in situ through encapsulation of the peat fibers with a layer of silicate. The results could increase strength and offer some resistance to biological and chemical oxidation. The silicification process involves the addition of three components: a cationic surfactant, a binding agent (a sodium metasilicate solution), and molasses to stimulate microbial fermentation, leading to the production of organic acids that will harden the geopolymer gel formed by the surfactant and binding agent. They achieved strengths of up to 1 MPa (UCS) in the laboratory using this process.

The main aim of this study was to determine the effect of cement–sodium silicate grout compounds on the void ratio (e) and the coefficient of secondary compression (C_s) of fibrous peats. Different ratios of grouts were tested in order to find the effect of each compound on the mentioned properties of fibrous peat.

### Materials and Methods

Fibrous peat was collected from different locations in Kampung Jawa, Kelang, southwest of Kuala Lumpur, Malaysia. The physicochemical properties of fibrous peat are presented in Table 1. Ordinary Portland cement, kaolinite [Al_2Si_2O_5(OH)_4], sodium silicate, and calcium chloride were used to mitigate settlement problems in fibrous peat.

In order to determine the effects of different compounds on the compressibility parameters of fibrous peat, samples were prepared by mixing a specified amount (as a percentage) of cement, kaolinite, sodium silicate, and calcium chloride by weight of wet peat. The dosages of the various compounds used

<table>
<thead>
<tr>
<th>Parameters</th>
<th>Method</th>
<th>Fibrous Peat</th>
</tr>
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<tbody>
<tr>
<td>Fiber content, %</td>
<td>ASTM D1997-91 [22]</td>
<td>79.1</td>
</tr>
<tr>
<td>Moisture content, %</td>
<td>BS 1377: Part 2: 1990, Clause 3 [23]</td>
<td>506.5</td>
</tr>
<tr>
<td>Specific gravity</td>
<td>BS 1377: Part 2: 1990, Clause 8.4 [23]</td>
<td>1.26</td>
</tr>
<tr>
<td>Organic content, %</td>
<td>BS 1377: Part 3: 1990, Clause 4 [23]</td>
<td>94.23</td>
</tr>
<tr>
<td>Bulk unit weight, kN/m³</td>
<td>BS 1377: Part 2: 1990, Clause 7 [23]</td>
<td>9.86</td>
</tr>
<tr>
<td>Degree of humification, %</td>
<td>von Post [24]</td>
<td>H_3</td>
</tr>
</tbody>
</table>
Results and Discussion

INFLUENCE OF ORDINARY PORTLAND CEMENT ON FIBROUS PEAT

The variation in secondary compression for 20 % and 30 % cement and effective pressures ranging from 25 to 200 kPa is shown in Fig. 2. It was observed that the coefficient of secondary compression $C_a$ decreased with the increase in cement. The $C_a$ of untreated peat was 0.122 at a pressure of 25 kPa. It decreased to 0.00023 and 0.00008 with 20 % cement, and it decreased to 0.00008 with 30 % cement at the same pressure. Further, $C_a$ was observed to increase with an increase in effective pressure. For untreated peat, it increased from 0.122 to 0.187 when the pressure was increased from 25 to 200 kPa. Similarly, it increased from 0.00023 to 0.00052 when the effective pressure increased from 25 to 200 kPa for samples with 20 % cement and from 0.00008 to 0.0002 for the same increase in effective pressure for samples with 30 % cement.

The variation in void ratio $e$ with an increase in cement content is shown in Fig. 3. The void ratio was observed to decrease with an increase in cement content. The void ratio, at an effective pressure of 10 kPa, was observed to decrease from 0.805 to 0.62 when the cement content increased from 20 % to 30 %. For samples with 20 % cement, it decreased from 0.805 to 0.79 when the effective pressure increased from 10 to 200 kPa. Similarly, for samples with 30 % cement, it decreased from 0.62 to 0.61 when the effective pressure increased from 10 to 200 kPa. The void ratio was also observed to decrease with an increase in effective pressure for all samples.

Pozzolanic reaction is responsible for the long-term strength gain of stabilized soil, so in this case the pozzolanic reaction might not be the main factor, because the samples were prepared by first mixing thoroughly homogenized peat at its natural water content with a household mixer and then adding the desired amounts of kaolinite, calcium chloride, cement, and sodium silicate. Three samples were prepared according to the percent weight of wet peat, as per the ratio of additives, and the average values of the results are reported. The samples were transferred to cylindrical containers and kept in a tray containing water. The compressibility parameters of fibrous peats, in vertical and horizontal directions, were evaluated with a Rowe cell according to BS 1377 Part 6: 1990 [23]. The Rowe cell apparatus is shown in Fig. 1.

FIG. 1 Rowe cell apparatus.
treated for less than a week. The ion exchange between calcium ions from cement and calcium chloride from kaolinite also would have decreased the mentioned parameters of the samples.

**INFLUENCE OF KAOLINITE ON FIBROUS PEAT**

The variation of $C_a$ with effective pressure for samples with 10 %, 20 %, and 30 % kaolinite is shown in Fig. 4. $C_a$ was observed to gradually decrease as the percentage of kaolinite increased.

The $C_a$ of untreated peat was 0.123 at an effective pressure of 25 kPa. With the addition of 0 %, 20 %, and 30 % kaolinite, it decreased to 0.0009, 0.00022, and 0.00007, respectively. Similarly, the $C_a$ of untreated peat increased with an increase in effective pressure from 0.123 at 25 kPa to 0.187 at 200 kPa.

Similar variation in $C_a$ was observed for samples with 0 %, 20 %, and 30 % kaolinite when the pressure was increased from 25 to 200 kPa.

The variation in void ratio with an increase in kaolinite is shown in Fig. 5. The void ratio was observed to decrease with an increase in kaolinite and with an increase in effective pressure from 10 kPa to 200 kPa.

The void ratio was found to be 1.1, 0.69, and 0.62 for samples with 10 %, 20 %, and 30 % kaolinite at an effective pressure of 10 kPa. The void ratio decreased with an increase in effective pressure for all samples. For samples with 10 % kaolinite, the void ratio decreased from 1.1 to 1.04 when the effective pressure increased from 10 to 200 kPa. However, for samples with more kaolinite, the void ratio showed a very small reduction, from 0.69 to 0.68 and from 0.62 to 0.61 for samples with 20 % and

**FIG. 3**
Influence of cement concentration on void ratio of fibrous peat.

**FIG. 4**
Influence of kaolinite concentration on coefficient of secondary compression of fibrous peat.
30% kaolinite, respectively. The effect of kaolinite on the compressibility behavior is considered small relative to the effect of cement.

These results agree well with the published results; with the addition of kaolinite and calcium chloride, the adsorption of organic compounds will increase and calcium may create a bridge or connection (mentioned above) between the organic compound and the mineral part of fibrous peat and kaolinite [9,10].

INFLUENCE OF SODIUM SILICATE ON FIBROUS PEAT

The variation in $C_a$ with changes in sodium silicate content and effective pressure is shown in Fig. 6.

As observed from Fig. 6, the $C_a$ decreased from 0.0008 to 0.0001 when the sodium silicate content increased from 0 to 2.5% at an effective pressure of 25 kPa. With an increase in sodium silicate content from 2.5% to 5%, a reversal in $C_a$ was observed. It increased from 0.0001 to 0.0003 at the same pressure.

The void ratio, as shown in Fig. 7, decreased from 1.04 to 0.62 when sodium silicate was increased from 0 to 2.5%. With a further increase in sodium silicate content, the void ratio increased, instead of decreasing, from 0.62 to 0.91, indicating that the optimum dose of sodium silicate is 2.5%.

This behavior could be due to the fact that the mixture of cement and sodium silicate in the presence of calcium chloride causes intensive hydration, and the OH$^-$ ions passing into the solution are consumed in the reaction of depolymerization and hydrolysis of silicate anions of the additives. When the concentration of calcium and hydroxide ions reaches a certain value,
calcium hydroxides crystallize out of solution and finally lead to the production of calcium silicate hydrate (C–S–H), which improves the compressibility parameters of fibrous peat [14,15].

A polymerization process takes place to form a gel when sodium silicate is mixed with soil. This gel makes the binder behave as a glue to bond the soil particles together and as a filler to reduce the void ratio of the soil.

Conclusions

Based on the results, the following conclusions can be drawn:

- Settlement, \( C_n \), and \( e \) decreased gradually with an increase in cement and kaolinite content. The effect of increased cement content was more significant than the effect of increased kaolinite content.
- All the parameters measured also decreased with an increasing percentage of sodium silicate binder until an optimum value was achieved. After the optimum percentage, an increase in the sodium silicate content increased the compressibility parameters.
- Regarding the effects of cement and sodium silicate (within 3%) on the samples, the research showed that when the percentages of these compounds were increased, the compressibility parameters of fibrous peat improved. This happens because the hydration of cement and the fast reaction between cement and sodium silicate causes hydrated calcium silicates to be formed, both via precipitation of silicate ions of the additive and via release of silicate and aluminate ions from the clinker.

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References


